

THEORETICAL AND MATHEMATICAL MODEL TESTS OF DESIGNS
OF TWO SUSPENDED SEDIMENT SAMPLING DEVICES

Prepared for

USDA FOREST SERVICE
ROCKY MOUNTAIN FOREST AND RANGE
EXPERIMENT STATION
Flagstaff, Arizona

Cooperative Research Agreement
No. 16-872-CA



Prepared by

Civil Engineering Department
Engineering Research Center
Colorado State University
Fort Collins, Colorado

D. B. Simons
R. M. Li
G. O. Brown

July 1983

CER82-83DBS-RML-GOB50

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AUTHORIZATION

This research was sponsored by the USDA Forest Service, Rocky Mountain Forest and Range Experiment Station and supported with Colorado State University matching funds. The investigations were conducted in accordance with the Cooperative Agreement No. 16-872-CA between the Rocky Mountain Forest and Range Experiment Station and Colorado State University. D. Ross Carder and Lawrence D. Garrett were the authorized project leaders for the Rocky Mountain Forest and Range Experiment Station and Daryl B. Simons and Ruh-Ming Li were the principal investigators for Colorado State University.

In accordance with the study plan, the report on the Theoretical and Mathematical Model Tests of Designs of Two Suspended Sediment Sampling Devices is submitted.

TABLE OF CONTENTS

<u>Section</u>	<u>Page</u>
AUTHORIZATION	ii
LIST OF FIGURES	iv
INTRODUCTION	1
DATA INPUT	3
RESULTS OUTPUT	7
EXAMPLE RUNS	10
PROGRAM APPLICATION	59
CONCLUSIONS AND RECOMMENDATIONS	61
APPENDIX A - Program Theory	63
APPENDIX B - Program Listing	82
REFERENCES	96

LIST OF FIGURES

<u>Figure</u>		<u>Page</u>
1	Box sampler dimensions	5
2	Pump sampler dimensions	5
A.1	Typical channel cross section	63
A.2	Incremental areas in a cross section	63
A.3	Subsection area	64
A.4	Sediment size distribution	68
A.5	Comparison between Einstein and Lane and Kalinske's sediment concentrations	73
A.6	Box sampler	77
A.7	Point sampler	77

INTRODUCTION

Study of water and sediment yields from forested watersheds is a primary task of research personnel at the Rocky Mountain Forest and Range Experiment Station, Flagstaff, Arizona. As an aid to this task, numerous water and sediment sampling sites have been constructed on selected watersheds throughout the Beaver Creek Watershed. Some sediment sampling sites utilize unique intake structures designed by Norman Champagne. One design is a converging section in the downstream direction that narrows the vertical axis. The other design is an upright pipe with a horizontally converging intake. Both designs are assumed to vertically integrate suspended sediment to provide a representative sample. However, sufficient field measurements have not been taken to verify this because of a variability and timing of significant runoff events. Since much of the suspended sediment information is collected using these samplers, it is important to have a thorough understanding of their response to different factors. Therefore, to ascertain the operating characteristics of these two suspended sediment sampling intakes, this study was undertaken so that development of a computer-based mathematical model could be used to "test" the two samplers for selected conditions. Collection of sufficient field data is necessary to verify the model. However, the model provides a sound basis for evaluation and qualitative comparisons of intake designs. These comparisons will enable redesign, if needed, of the current intake structures. Verification of the intake designs and the mathematical model can best be achieved based upon extensive field data collection over a range of flow conditions. The verification of designs is a topic for latter studies and would be of limited value unless actual field conditions were modeled.

The model developed is based on the physical processes of open channel flow and sediment transport. The model is general in application and can be used for channels of different geometry, slope, bed material, flow discharge, and flow frequencies. It accounts for sampler geometry, location, and mixing conditions in the sampler.

The following sections describe data input, results output, model applications, conclusions and recommendations. The appendices contain the program theory and listing.

DATA INPUT

General

The program SAMPLER is written in FORTRAN IV and is self contained except for requiring the IMSL subroutine MDNOR. All input units are in feet and seconds. All data are input from the main program sampler, with the exception of the sampler dimensions which are defined with DATA statements in subroutines BOX and PUMP. The program has been written to be self documenting, thus these instructions have been kept to a minimum for the sake of clarity. The user is referred to the program listing for more details.

Required Data

The program requires the following data:

1. A digitized channel cross section at the sampler location.
2. The median (D_{50}) bed material size at each cross section point.
3. The Manning's n value at each cross section point.
4. A water discharge-frequency relationship at the sampler location.
5. The slope of the energy grade line at the sampler location.
6. The Darcy-Weisbach grain roughness at the section.
7. The sediment sizes of interest.
8. The sediment sampler dimensions.

Input Formats

The following cards give the input data for program execution. The input is read from TAPE5.

<u>Card Number</u>	<u>Format</u>	<u>Description</u>
1	(12A5)	Title, Title of Run
2	(4I5, 2F10.4)	IR, Sampler number (1 for BOX, 2 for PUMP) ND, Number of points in the cross section NQ, Number of discharges NS1Z, Number of particle size fractions S, the slope of the energy grade line FO, the Darcy-Weisbach grain roughness
3 (Repeat card for each of NQ discharge)	(2F10.2)	Q(J), Discharge(J) in cfs Freq(J), Frequency of discharge(J) in decimal fraction
4 (Repeat card for each of ND cross section points)	(4F10.4)	X(K), Cross section point(K) horizontal distance Z(K), point(K) vertical elevation F(K), point(K) Manning's n value D(K), point(K) median (D_{50}) sediment size
5 (Repeat card for each of NS1Z+1 size fractions)	(F10.6)	DBD(I), Lower size limit of sediment range (I), last card upper size limit of fraction NS1Z

Data Statements

Data statements are used to define the sampler dimensions to allow the most flexibility in modifying sampler designs. Figures 1 and 2 show the sampler dimensions for the box and pump sampler.

The following DATA statements are in the program. The user should change these values to fit needs.

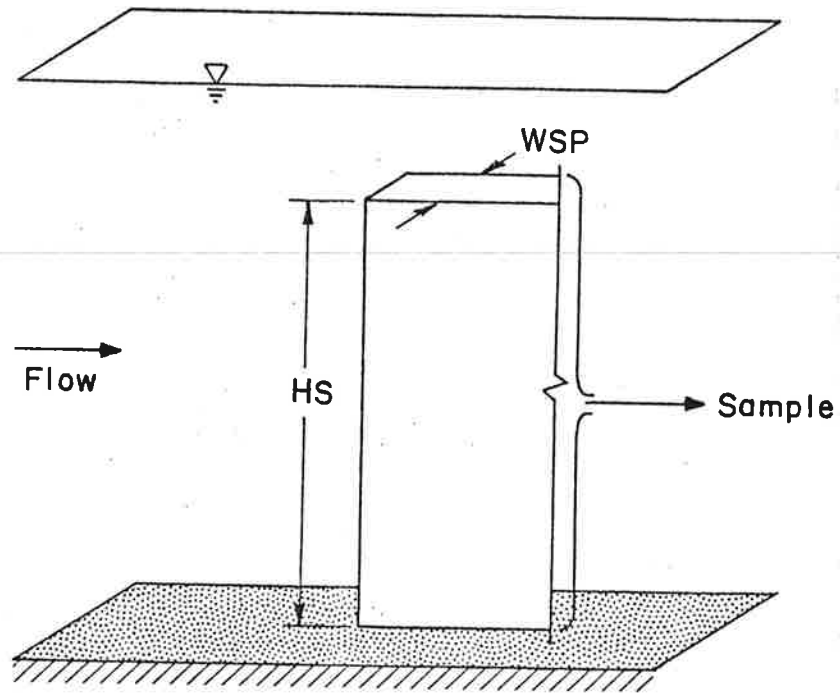


Figure 1. Box sampler dimensions.

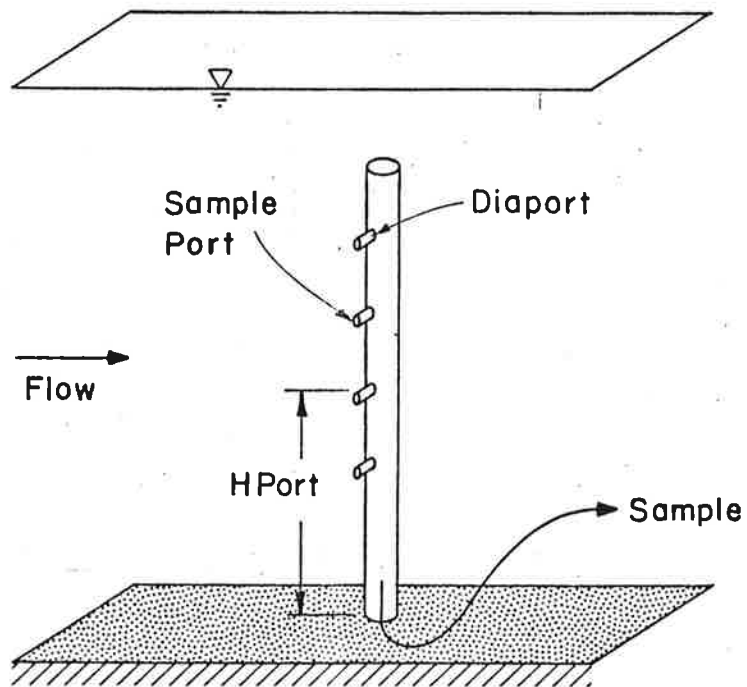


Figure 2. Pump sampler dimensions.

Subroutine BOX

DATA WSP/0.382/ (width of sampler)

DATA HS/1.5/ (height of sampler)

Subroutine PUMP

DATA NPORT/5/ (number of intake ports)

DATA DIAPORT/0.02/ (diameter of ports)

DATA HPORTS/0.5,1.0,1.5,2.0,2.5/ (height of ports)

RESULTS OUTPUT

General

All program output is printed on TAPE6. The printed output results are by and large self explanatory. Examples of the complete results for both the box and pump samplers are contained in the next section. This section will describe in general the results. The output has three main parts: 1) Input and Initial Calculations, 2) Single Discharge Flow, Transport and Sampling Calculations, and 3) Overall Sampling Calculations.

Input and Initial Calculations

The input data is listed out for user checking along with initial calculations of mean sediment size for each size range, the fall velocity for each size range, and the subsection particle size distribution. A subsection is the channel between two cross section points.

Single Discharge Flow, Transport and Sampling Calculations

For each discharge, five tables are printed out. The tables are:

1. Flow Properties, which contain the subsection discharge, velocity and depth of flow.
2. Sediment Transport by Size Fraction, which contains the total transport by size fraction.
3. Sediment Transport by Subsection and Size Fraction, which lists the sediment transport of each size fraction in each subsection. This table is useful in determining where in the section sediment is being transported.

4. Efficiency of Sampler by Subsection and Size Fraction, which lists the decimal percent of each size fraction the sampler would catch if it were placed in that subsection. This value ranges from 0.0 to 1.0. A value of 1.00 is the best. The efficiency of a sampler is also reduced by the amount of the sampler out of the water. Thus a pump sampler with only 2 of 5 ports out of the water will have a 0.40 efficiency at best. An efficiency of -1.0 shows that no sediment would be transported or sampled in that subsection.
5. Ratio of Sampled Sediment Concentration to Average by Subsection and Size Fraction, which lists the ratio of the sampled concentration to the total concentration at the cross section as if the sampler were placed at that section. This value ranges from zero to ten or higher. A value of 1.0 for each size fraction would indicate perfect sampling.

Overall Sampling Calculations

The overall sampling calculation output contains two tables. The tables are:

1. Overall Flow Frequency Weighted, Ratio of Sampled Sediment Concentration to Average, by Subsection and Size Fraction. This table lists the ratio of the sum sampled sediment for each flow multiplied by the flow frequency, and the total sediment transport for each flow multiplied by its frequency. The value should range from zero to two or more. A value of 1.0 would be best.

2. Overall Flow Frequency Weighted Ratio of Sampled Concentration to Average by Subsection. This table lists ratio of the sum of the total sampled concentration multiplied by the flow frequency, and the sum of the total sediment transport multiplied by flow frequency, if the sampler were placed in the subsection. The value will range from zero to two or more. A value of 1.0 is best.

Error Output

The program has only one error output. This output occurs when the program is unable to calculate the depth of flow. When it occurs, the following message is printed,

DOES NOT CONVERGE WITHIN 10 ITERATIONS

If this happens the user should check the input data.

EXAMPLE RUNS

Box Sampler Run

An example run was performed for the box sampler. A listing of the input and output follow. The example uses generated data that is representative of conditions in National Forests. Page 11 shows the example input data. The cross section is roughly triangular in shape with a total top width of 60 feet. Five flow discharge are used, 1000, 500, 250, 100 and 50 cfs while the flow frequencies are 0.01, 0.05, 0.10, 0.50 and 0.34 respectively. The energy slope has a relatively high value of 0.024 ft/ft, and the grain roughness is 0.25. The bed material of interest is broken into 10 size fractions ranging from 0.000051 to 0.105 feet. The medium bed material at the cross section points ranges from 0.02 to 0.15 feet. The high value of 0.15 feet at horizontal distance 39 feet was used to show the effect of a gravel bar on the calculations. Note that this size is larger than the particle size fractions used. This causes no computational problem in the program but the transport rates and sampling of sizes larger than the upper fraction are not calculated.

Pages 12 to 34 show the example output. In this example the best location for the sampler would be in subsections 9 or 12. Reviewing the "Over All" tables (page 34) shows a sampler located in those subsections would underpredict the smallest fraction by a ratio of 0.31 and underpredict the largest fraction by a ratio of 0.49. The total sediment transport would be overpredicted by a ratio of 1.13.

Box Sampler Example Input (see page 4 for explanation)

Card Number	EXAMPLE RUN FOR PROGRAM SAMPLER WITH BOX SAMPLER
1	1000.00 10 0.01 0.0200
2	1000.00 10 0.01 0.0200
3	1000.00 10 0.01 0.0200
4	1000.00 10 0.01 0.0200
5	1000.00 10 0.01 0.0200

Box Sampler Example Output

S A M P L E R
 SEDIMENT SAMPLER EVALUATION PROGRAM
 DEVELOPED BY G.O. BROWN AND R.M. LI
 AT COLORADO STATE UNIVERSITY
 FOR THE USDA FOREST SERVICE
 ROCKY MOUNTAIN FOREST AND RANGE EXPERIMENT STATION
 FLAGSTAFF, ARIZONA

 *
 * EXAMPLE RUN FOR PROGRAM SAMPLER WITH BOX SAMPLER
 *

SAMPLER NUMBER = 1

NUMBER OF CROSS SECTION POINTS = 21
 NUMBER OF DISCHARGES = 5
 NUMBER OF SEDIMENT SIZES = 10
 FRICTION SLOPE = .02466 FT/FT
 GRAIN ROUGHNESS = .25000

CROSS SECTION		POINT	HORIZONTAL	VERTICAL	N	D50
1	0.00	13.50	.020	.020000		
2	3.00	13.20	.020	.020000		
3	5.00	12.90	.020	.020000		
4	7.00	12.50	.020	.020000		
5	12.00	11.80	.030	.015000		
6	18.00	11.40	.030	.015000		
7	21.00	11.10	.030	.015000		
8	24.00	10.70	.030	.015000		
9	27.00	10.40	.030	.010000		
10	30.00	10.00	.030	.010000		
11	33.00	10.40	.030	.015000		
12	36.00	10.70	.030	.015000		
13	39.00	11.10	.030	.015000		
14	42.00	11.40	.030	.015000		
15	45.00	11.80	.030	.015000		
16	48.00	12.10	.030	.015000		
17	51.00	12.50	.030	.015000		
18	54.00	12.90	.030	.015000		
19	57.00	13.20	.030	.015000		
20		13.50	.030	.015000		
21			.030	.015000		

Box Sampler Example Output (cont.)

SEDIMENT PROPERTIES									
SUB SECTION	1	2	3	4	PARTICLE SIZE FRACTION				
					5	6	7	8	9 10
MEAN SEDIMENT SIZE IN FEET									
.1022E-03	.2899E-03	.5798E-03	.1160E-02	.2319E-02	.4639E-02	.9270E-02	.1856E-01	.3716E-01	.7425E-01
FALL VELOCITY IN FT/SEC									
.2799E-02	.2153E-01	.6918E-01	.1536E+00	.2396E+00	.3913E+00	.5659E+00	.8073E+00	.1145E+01	.1621E+01
SUBSECTION	PARTICLE SIZE DISTRIBUTION								
1	.0000	.0000	.0000	.00015	.00440	.04934	.21630	.38241	.26443
2	.0000	.0000	.0000	.00015	.00440	.04934	.21630	.38241	.26443
3	.0000	.0000	.0000	.00015	.00440	.04934	.21630	.38241	.26443
4	.0000	.0000	.0000	.00015	.00440	.04934	.21630	.38241	.26443
5	.0000	.0000	.0000	.00033	.00785	.07247	.30515	.35845	.21858
6	.0000	.0000	.0000	.00059	.01344	.10225	.30515	.35845	.17359
7	.0000	.0000	.0000	.00069	.01344	.10225	.30515	.35845	.17359
8	.0000	.0000	.0000	.00181	.02681	.15520	.35845	.35845	.11732
9	.0000	.0000	.0000	.00440	.04934	.21762	.35845	.35845	.07313
10	.0000	.0000	.0000	.00440	.04934	.21762	.35845	.35845	.07313
11	.0000	.0000	.0000	.00103	.02681	.15520	.35845	.35845	.07313
12	.0000	.0000	.0000	.00003	.00006	.00210	.02954	.16572	.31599
13	.0000	.0000	.0000	.00000	.00006	.00210	.02954	.16572	.31599
14	.0000	.0000	.0000	.00000	.00006	.00210	.02954	.16572	.31599
15	.0000	.0000	.0000	.00033	.00785	.07247	.30515	.35845	.21858
16	.0000	.0000	.0000	.00059	.01344	.10225	.30515	.35845	.17359
17	.0000	.0000	.0000	.00069	.01344	.10225	.30515	.35845	.17359
18	.0000	.0000	.0000	.00181	.02681	.15520	.35845	.35845	.11732
19	.0000	.0000	.0000	.00440	.04934	.21762	.35845	.35845	.07313
20	.0000	.0000	.0000	.00440	.04934	.21762	.35845	.35845	.07313

Box Sampler Example Output (cont.)

DISCHARGE, 1 1000.0000 CFS WITH .0100 FREQUENCY	
FLOW PROPERTIES	
SUB SECTION	DISCHARGE CFS
1	0.0000
2	.8744
3	8.3065
4	22.5041
5	33.5639
6	43.7934
7	52.6018
8	57.6354
9	107.5115
10	132.1255
11	107.5116
12	83.6364
13	62.5018
14	43.7934
15	33.5639
16	22.5041
17	8.3065
18	.8744
19	0.0000
20	0.0000

VELOCITY FT/SEC		DEPTH FT	
1	0.0000	0.0000	0.0000
2	3.0571	.1381	.4262
3	6.4970	.7762	1.1262
4	9.6646	1.4262	1.8262
5	9.9345	1.8262	2.5262
6	9.8903	1.8262	2.8762
7	11.4268	2.5262	2.8762
8	12.8109	2.5262	2.8762
9	14.1864	2.5262	2.8762
10	15.4287	2.5262	2.8762
11	16.6364	2.5262	2.8762
12	17.8109	2.5262	2.8762
13	18.9345	2.5262	2.8762
14	19.8903	2.5262	2.8762
15	20.6646	2.5262	2.8762
16	21.4287	2.5262	2.8762
17	22.1781	2.5262	2.8762
18	22.8109	2.5262	2.8762
19	23.4287	2.5262	2.8762
20	24.0000	2.5262	2.8762

TOTAL SEDIMENT TRANSPORT = 87.6487 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.9104E-06	.1765E-03	.1213E-01	.3056E+00	.3157E+01	.1409E+02	.2770E+02	.2582E+02	.1229E+02	.4249E+01

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

[illegible]

[illegible]

Box Sampler Example Output (cont.)

RATIO OF SAMPLED SEDIMENT CONCENTRATION TO AVERAGE, BY SUBSECTION AND SIZE FRACTION

SUB SECTION	1	2	3	4	5	6	7	8	9	10
1	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
2	0.01174	0.00399	0.01116	0.03043	0.00047	0.00000	0.00000	0.00000	0.00000	0.00000
3	0.03581	0.00399	0.02144	0.07685	0.13500	0.00000	0.00000	0.00000	0.00000	0.00000
4	0.03246	0.00399	0.02026	0.13887	0.18573	0.00000	0.00000	0.00000	0.00000	0.00000
5	0.05820	0.00399	0.01569	0.23851	0.26821	0.00000	0.00000	0.00000	0.00000	0.00000
6	0.08567	0.00399	0.02510	0.33713	0.33630	0.00000	0.00000	0.00000	0.00000	0.00000
7	0.14931	0.00399	0.05517	0.42733	0.37035	0.00000	0.00000	0.00000	0.00000	0.00000
8	0.24321	0.00399	0.09567	0.50971	0.36517	0.00000	0.00000	0.00000	0.00000	0.00000
9	0.41515	0.00399	0.16731	0.72738	0.51584	0.00000	0.00000	0.00000	0.00000	0.00000
10	0.61515	0.00399	0.27317	0.90971	0.61584	0.00000	0.00000	0.00000	0.00000	0.00000
11	0.80000	0.00000	0.40004	1.00000	0.78551	0.00000	0.00000	0.00000	0.00000	0.00000
12	0.93815	0.00000	0.54559	1.00000	0.96917	0.00000	0.00000	0.00000	0.00000	0.00000
13	0.98313	0.00000	0.67166	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
14	0.99683	0.00000	0.72022	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
15	0.99999	0.00000	0.75517	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
16	1.00000	0.00000	0.77145	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
17	1.00000	0.00000	0.78551	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
18	1.00000	0.00000	0.79551	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
19	1.00000	0.00000	0.80000	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000
20	1.00000	0.00000	0.80000	1.00000	1.00000	0.00000	0.00000	0.00000	0.00000	0.00000

Box Sampler Example Output (cont.)

DISCHARGE, 2		500.0000 CFS WITH		.0500 FREQUENCY	
FLOW PROPERTIES					
SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT		
1	0.0000	0.0000	0.0000		
2	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000		
4	1.1519	3.2752	.1557		
5	1.4411	5.4393	.1557		
6	17.3342	8.6084	.8063		
7	19.2261	10.4235	1.1563		
8	25.3269	11.5519	1.3563		
9	45.3277	12.5287	2.2063		
10	85.5719	11.5519	1.8563		
11	45.3277	11.0242	1.5063		
12	29.2351	8.6084	1.1563		
13	17.3342	5.4393	.4351		
14	1.1519	3.2752	0.0000		
15	0.0000	0.0000	0.0000		
16	0.0000	0.0000	0.0000		
17	0.0000	0.0000	0.0000		
18	0.0000	0.0000	0.0000		
19	0.0000	0.0000	0.0000		
20	0.0000	0.0000	0.0000		

TOTAL SEDIMENT TRANSPORT = 36.5720 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	7	8	9	10
.4737E-06	.9052E-04	.6071E-02	.1492E+00	.1510E+01	.6502E+01	.1207E+02	.1026E+02	.4498E+01 .1583E+01

[illegible]

Box Sampler Example Output (cont.)

DISCHARGE, 3 250.0000 CFS WITH			.1000 FREQUENCY	
FLOW PROPERTIES				
SUR SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT	
1	0.0000	0.0000	0.0000	
2	0.0000	0.0000	0.0000	
3	0.0000	0.0000	0.0000	
4	0.0000	0.0000	0.0000	
5	0.0000	0.0000	0.0000	
6	2.6301	1.0103	.0355	
7	10.4294	3.2108	.2730	
8	21.8674	5.5793	.6230	
9	36.5855	7.4910	.9730	
10	53.3619	9.2175	1.3230	
11	72.5875	10.7512	1.6730	
12	94.3674	12.2175	1.9730	
13	118.6284	13.4310	2.2730	
14	145.4381	14.5793	2.5730	
15	174.6301	15.6103	2.8730	
16	206.2270	16.6103	3.1730	
17	240.0000	17.5793	3.4730	
18	276.0000	18.5000	3.7730	
19	314.0000	19.3750	4.0730	
20	354.0000	20.2000	4.3730	

TOTAL SEDIMENT TRANSPORT = 15.9760 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.2400E-06	.4520E-04	.2967E-02	.7163E-01	.7173E+00	.3044E+01	.5483E+01	.4384E+01	.1734E+01	.5389E+00

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

[illegible]

Box Sampler Example Output (cont.)

[illegible]

[illegible]

Box Sampler Example Output (cont.)

DISCHARGE, 4 100.0000 CFS WITH .5000 FREQUENCY		
FLOW PROPERTIES		
SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC DEPTH FT
1	0.0000	0.0000
2	0.0000	0.0000
3	0.0000	0.0000
4	0.0000	0.0000
5	0.0000	0.0000
6	0.0000	0.0000
7	0.0000	0.0000
8	0.0000	0.0000
9	0.0000	0.0000
10	0.0000	0.0000
11	0.0000	0.0000
12	0.0000	0.0000
13	0.0000	0.0000
14	0.0000	0.0000
15	0.0000	0.0000
16	0.0000	0.0000
17	0.0000	0.0000
18	0.0000	0.0000
19	0.0000	0.0000
20	0.0000	0.0000

TOTAL SEDIMENT TRANSPORT = 5.3698 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.9374E-07	.1733E-04	.1111E-02	.2649E-01	.2651E+00	.1118E+01	.1962E+01	.1455E+01	.4591E+00	.8372E-01

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

[illegible]

[illegible]

Box Sampler Example Output (cont.)

DISCHARGE, 5 50.0000 CFS WITH .3400 FREQUENCY

FLOW PROPERTIES

SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT
1	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000
4	0.0000	0.0000	0.0000
5	0.0000	0.0000	0.0000
6	0.0000	0.0000	0.0000
7	0.0000	0.0000	0.0000
8	0.0000	0.0000	0.0000
9	0.0000	0.0000	0.0000
10	0.0074	2.3368	0.1695
11	5.9655	4.7475	.4891
12	17.0830	6.7865	.8391
13	17.0830	6.7865	.8391
14	1.0074	2.3368	.1695
15	0.0000	0.0000	0.0000
16	0.0000	0.0000	0.0000
17	0.0000	0.0000	0.0000
18	0.0000	0.0000	0.0000
19	0.0000	0.0000	0.0000
20	0.0000	0.0000	0.0000

TOTAL SEDIMENT TRANSPORT = 2.3342 CFS.

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.4435E-07	.8088E-05	.5107E-03	.1216E-01	.1222E+00	.5131E+00	.8829E+00	.6213E+00	.1659E+00	.1616E-01

[illegible]

Box Sampler Example Output (cont.)

OVER ALL FLOW FREQUENCY WEIGHTED, RATIO OF SAMPLED SEDIMENT CONCENTRATION TO AVERAGE,
BY SUBSECTION AND SIZE FRACTION

SUB SECTION	1	2	3	4	PARTICLE SIZE FRACTION		7	8	9	10
1	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
2	0.00013	0.00035	0.00053	0.00267	0.00030	0.00030	0.00030	0.00030	0.00030	0.00030
3	0.00028	0.00071	0.00179	0.00467	0.00122	0.00122	0.00122	0.00122	0.00122	0.00122
4	0.00078	0.00200	0.00514	0.01365	0.00334	0.00334	0.00334	0.00334	0.00334	0.00334
5	0.00365	0.00764	0.01539	0.03700	0.00930	0.00930	0.00930	0.00930	0.00930	0.00930
6	0.01320	0.04333	0.07441	0.12323	0.02151	0.02151	0.02151	0.02151	0.02151	0.02151
7	0.03762	0.06456	0.11042	0.18895	0.05122	0.05122	0.05122	0.05122	0.05122	0.05122
8	0.13742	0.40066	0.52284	0.67843	0.08145	0.08145	0.08145	0.08145	0.08145	0.08145
9	0.37422	2.11386	2.11559	2.10050	0.09333	0.09333	0.09333	0.09333	0.09333	0.09333
10	0.07663	0.40066	0.52284	0.67843	0.05333	0.05333	0.05333	0.05333	0.05333	0.05333
11	0.00000	0.00000	0.00000	0.00016	0.00135	0.00135	0.00135	0.00135	0.00135	0.00135
12	0.00000	0.00000	0.00000	0.00011	0.00052	0.00052	0.00052	0.00052	0.00052	0.00052
13	0.00355	0.02597	0.04369	0.07416	0.00452	0.00452	0.00452	0.00452	0.00452	0.00452
14	0.00078	0.02597	0.04369	0.07416	0.00930	0.00930	0.00930	0.00930	0.00930	0.00930
15	0.00028	0.00200	0.00514	0.01365	0.00334	0.00334	0.00334	0.00334	0.00334	0.00334
16	0.00013	0.00071	0.00179	0.00467	0.00122	0.00122	0.00122	0.00122	0.00122	0.00122
17	0.00000	0.00035	0.00053	0.00267	0.00030	0.00030	0.00030	0.00030	0.00030	0.00030
18	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
19	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
20	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000

OVER ALL FLOW FREQUENCY WEIGHTED, RATIO OF SAMPLED CONCENTRATION TO AVERAGE,
BY SUBSECTION

SUBSECTION	RATIO
1	0.00000
2	0.05184
3	0.10368
4	0.20736
5	0.31104
6	0.41472
7	0.51840
8	0.62208
9	0.72576
10	0.82944
11	0.93312
12	1.03680
13	1.14048
14	1.24416
15	1.34784
16	1.45152
17	1.55520
18	1.65888
19	1.76256
20	1.86624

Pump Sampler Run

An example run was performed for the pump sampler. Its input is identical to the box sampler input except for the title card. Pages 36 to 58 show the example output. In this example, the best location for the sampler would be in subsections 10 or 11. Reviewing the overall tables (page 58) shows a sampler located in those subsections would overpredict the smallest size fractions by a ratio of 2.99 and not measure any of the two largest size fractions. The total sediment transport would be underpredicted by a ratio of 0.44.

Pump Sampler Example Output

S A M P L E R
 SEDIMENT SAMPLER EVALUATION PROGRAM
 DEVELOPED BY G.O. BROWN AND R.M. LI
 AT COLORADO STATE UNIVERSITY
 FOR THE USDA FOREST SERVICE
 ROCKY MOUNTAIN FOREST AND RANGE EXPERIMENT STATION
 FLAGSTAFF, ARIZONA

 * EXAMPLE RUN FOR PROGRAM SAMPLER WITH MULTIPLE PORTS *
 * *****

SAMPLER NUMBER = 2

NUMBER OF CROSS SECTION POINTS = 21
 NUMBER OF DISCHARGES = 5
 NUMBER OF SEDIMENT SIZES = 10
 FRICTION SLOPE = .02300 FT/FT
 GRAIN ROUGHNESS = .25000

CROSS SECTION		POINT		HORIZONTAL	VERTICAL	N	D50
1	0.00	13.50	.020	.020000			
2	3.00	13.20	.020	.020000			
3	6.00	12.90	.020	.020000			
4	9.00	12.50	.020	.020000			
5	12.00	12.10	.030	.015000			
6	15.00	11.80	.030	.015000			
7	18.00	11.40	.030	.015000			
8	21.00	11.10	.030	.015000			
9	24.00	10.70	.030	.015000			
10	27.00	10.40	.030	.010000			
11	30.00	10.00	.030	.010000			
12	33.00	10.70	.030	.015000			
13	36.00	11.10	.030	.015000			
14	39.00	11.40	.030	.015000			
15	42.00	11.80	.030	.015000			
16	45.00	12.10	.020	.020000			
17	48.00	12.50	.020	.020000			
18	51.00	12.80	.020	.020000			
19	54.00	13.20	.020	.020000			
20	57.00	13.50	.020	.020000			
21	60.00						

Pump Sampler Example Output (cont.)

SEDIMENT PROPERTIES											
SUB SECTION	1	2	3	4	PARTICLE SIZE FRACTION			7	8	9	10
MEAN SEDIMENT SIZE IN FEET											
	.1022E-03	.2899E-03	.5798E-03	.1160E-02	.2319E-02	.4639E-02	.9270E-02	.1856E-01	.3716E-01	.7425E-01	
FALL VELOCITY IN FT/SEC											
	.2759E-02	.2153E-01	.6918E-01	.1536E+00	.2596E+00	.3913E+00	.5659E+00	.8073E+00	.1145E+01	.1621E+01	
SUBSECTION PARTICLE SIZE DISTRIBUTION											
1	.0000	.0000	.0000	.0000	.0040	.0493	.2169	.3828	.2644	.0735	
2	.0000	.0000	.0000	.0000	.0040	.0493	.2169	.3828	.2644	.0735	
3	.0000	.0000	.0000	.0000	.0040	.0493	.2169	.3828	.2644	.0735	
4	.0000	.0000	.0000	.0000	.0040	.0493	.2169	.3828	.2644	.0735	
5	.0000	.0000	.0000	.0000	.00785	.07247	.26286	.36685	.21858	.05015	
6	.0000	.0000	.0000	.0000	.01344	.10225	.30615	.35685	.11723	.03286	
7	.0000	.0000	.0000	.0000	.01344	.10225	.30615	.35685	.11723	.03286	
8	.0000	.0000	.0000	.0000	.02344	.15522	.35430	.26693	.07313	.01690	
9	.0000	.0000	.0000	.0000	.04334	.21762	.38005	.26693	.07313	.00802	
10	.0000	.0000	.0000	.0000	.04334	.21762	.38005	.26693	.07313	.00802	
11	.0000	.0000	.0000	.0000	.02681	.15520	.35480	.26554	.07313	.01690	
12	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
13	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
14	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
15	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
16	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
17	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
18	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
19	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	
20	.0000	.0000	.0000	.0000	.00006	.00210	.02954	.16957	.31599	.03159	

Pump Sampler Example Output (cont.)

DISCHARGE		1	1000.0000 CFS WITH	.0100 FREQUENCY
FLOW PROPERTIES				
SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT	
1	0.0000	0.0000	0.0000	
2	.8744	3.0571	.1351	
3	8.3055	6.4970	.4262	
4	22.5041	9.5646	.7762	
5	33.5639	9.9345	1.1262	
6	43.7994	9.8903	1.4762	
7	53.6018	11.2668	1.9262	
8	63.5116	12.103	2.1762	
9	107.5116	14.1864	2.5262	
10	133.1256	15.4287	2.8762	
11	133.1256	15.4287	2.5262	
12	107.5116	14.1864	2.1762	
13	83.6018	12.8109	1.8262	
14	63.5116	11.4268	1.4762	
15	43.7994	9.8903	1.1262	
16	33.5639	9.5646	.7762	
17	22.5041	6.4970	.4262	
18	8.3055	3.0571	.1351	
19	.8744	0.0000	0.0000	
20	0.0000	0.0000	0.0000	

TOTAL SEDIMENT TRANSPORT = 87.6487 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.9104E-06	.1765E-03	.1213E-01	.3056E+00	.3167E+01	.1409E+02	.2770E+02	.2582E+02	.1229E+02	.4249E+01

Pump Sampler Example Output (cont.)

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

SUB SECTION	1	2	3	4	5	6	7	8	9	10
1	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
2	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
3	0.00000	0.00000	0.00000	0.00013	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
4	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
5	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
6	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
7	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
8	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
9	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
10	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
11	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
12	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
13	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
14	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
15	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
16	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
17	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
18	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
19	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351
20	0.00000	0.00000	0.00000	0.00014	0.00025	0.00038	0.00070	0.00143	0.00234	0.00351

Pump Sampler Example Output (cont.)

EFFICIENCY OF SAMPLER BY SUBSECTION AND SIZE FRACTION										
SUB SECTION	1	2	3	4	5	6	7	8	9	10
1	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000
2	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
3	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000
4	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000
5	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000
6	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000
7	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
8	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
9	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
10	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
11	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000
12	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000
13	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000
14	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000
15	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
16	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000	-1.00000
17	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
18	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000	0.20000
19	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000	0.40000
20	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000	0.60000
21	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000	0.80000
22	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
23	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
24	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
25	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
26	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
27	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000
28	1.00000	1.00000	1.00000	1.00						

Pump Sampler Example Output (cont.)

[illegible]

Pump Sampler Example Output (cont.)

DISCHARGE, 2 500.0000 CFS WITH .0500 FREQUENCY												
FLOW PROPERTIES												
SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT									
1	0.0000	0.0000	0.0000									
2	0.0000	0.0000	0.0000									
3	0.0000	0.0000	0.0000									
4	1.4451	3.2752	.1531									
5	7.4451	5.4393	.4563									
6	15.2261	6.6084	.8063									
7	29.2261	8.4255	1.1563									
8	45.2261	10.0242	1.8563									
9	64.5719	11.5519	2.2063									
10	85.5719	12.9287	2.8563									
11	64.5719	11.5519	1.8563									
12	45.5719	10.0242	1.1563									
13	29.2261	8.4255	.4563									
14	15.2261	6.6084	.1531									
15	7.4451	5.4393	0.0000									
16	1.4451	3.2752	0.0000									
17	0.0000	0.0000	0.0000									
18	0.0000	0.0000	0.0000									
19	0.0000	0.0000	0.0000									
20	0.0000	0.0000	0.0000									
TOTAL SEDIMENT TRANSPORT = 36.5720 CFS												
SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS												
1	2	3	4	5	6	7	8	9	10			
.4737E-06	.9052E-04	.6071E-02	.1492E+00	.1510E+01	.6502E+01	.1207E+02	.1026E+02	.4498E+01	.1583E+01			

[illegible]

Pump Sampler Example Output (cont.)

[illegible]

Pump Sampler Example Output (cont.)

DISCHARGE, 3		250.0000 CFS WITH		.1000 FREQUENCY	
FLOW PROPERTIES					
SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT		
1	0.0000	0.0000	0.0000		
2	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000		
4	0.0000	0.0000	0.0000		
5	.0270	1.0103	.0365		
6	2.6301	3.2108	.2730		
7	10.4284	5.5793	.6230		
8	21.8674	7.4910	.9730		
9	33.5855	9.2175	1.3230		
10	33.5855	10.7512	1.6730		
11	33.5855	9.2175	1.3230		
12	36.5855	7.4910	.9730		
13	21.8674	5.5793	.6230		
14	10.4284	3.2108	.2730		
15	2.6301	1.0103	.0365		
16	0.0000	0.0000	0.0000		
17	0.0000	0.0000	0.0000		
18	0.0000	0.0000	0.0000		
19	0.0000	0.0000	0.0000		
20	0.0000	0.0000	0.0000		

TOTAL SEDIMENT TRANSPORT = 15.9760 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.2400E-06	.4520E-04	.2967E-02	.7163E-01	.7173E+00	.3044E+01	.5483E+01	.4384E+01	.1734E+01	.5389E+00

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

[illegible]

[illegible]

Pump Sampler Example Output (cont.)

DISCHARGE, 4 100.0000 CFS WITH .5000 FREQUENCY

FLOW PROPERTIES

SUB SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH FT
1	0.0000	0.0000	0.0000
2	0.0000	0.0000	0.0000
3	0.0000	0.0000	0.0000
4	0.0000	0.0000	0.0000
5	0.0000	0.0000	0.0000
6	0.0000	0.0000	0.0000
7	0.0000	0.0000	0.0000
8	0.0000	0.0000	0.0000
9	0.0000	0.0000	0.0000
10	0.0000	0.0000	0.0000
11	0.0000	0.0000	0.0000
12	0.0000	0.0000	0.0000
13	0.0000	0.0000	0.0000
14	0.0000	0.0000	0.0000
15	0.0000	0.0000	0.0000
16	0.0000	0.0000	0.0000
17	0.0000	0.0000	0.0000
18	0.0000	0.0000	0.0000
19	0.0000	0.0000	0.0000
20	0.0000	0.0000	0.0000

TOTAL SEDIMENT TRANSPORT = 5.3698 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	6	7	8	9	10
.9374E-07	.1733E-04	.1111E-02	.2649E-01	.2651E+00	.1118E+01	.1962E+01	.1455E+01	.4591E+00	.8372E-01

SEDIMENT TRANSPORT BY SUBSECTION AND SIZE FRACTION, IN CFS

[illegible]

[illegible]

Pump Sampler Example Output (cont.)

RATIO OF SAMPLED SEDIMENT CONCENTRATION TO AVERAGE, BY SUBSECTION AND SIZE FRACTION

SUB SECTION	1	2	3	4	5	6	7	8	9	10
1	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
2	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
3	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
4	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
5	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
6	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
7	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
8	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
9	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
10	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
11	9.34020	1.10398	1.02593	3.65159	3.0102	0.09714	0.01850	0.0023	0.00000	0.00000
12	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
13	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
14	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
15	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
16	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
17	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
18	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
19	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000
20	9.34020	8.28264	6.16902	3.55771	1.733274	7.0360	22.113	0.0035	0.00000	0.00000

Pump Sampler Example Output (cont.)

DISCHARGE, 5		50.0000 CFS WITH		.3400 FREQUENCY	
FLOW PROPERTIES					
SUR SECTION	DISCHARGE CFS	VELOCITY FT/SEC	DEPTH ft		
1	0.0000	0.0000	0.0000		
2	0.0000	0.0000	0.0000		
3	0.0000	0.0000	0.0000		
4	0.0000	0.0000	0.0000		
5	0.0000	0.0000	0.0000		
6	0.0000	0.0000	0.0000		
7	0.0000	0.0000	0.0000		
8	1.0074	2.3368	.1695		
9	6.9656	4.7476	.4231		
10	17.0830	6.7865	.8391		
11	17.0830	6.7865	.8391		
12	6.9656	4.7476	.4231		
13	1.0074	2.3368	.1695		
14	0.0000	0.0000	0.0000		
15	0.0000	0.0000	0.0000		
16	0.0000	0.0000	0.0000		
17	0.0000	0.0000	0.0000		
18	0.0000	0.0000	0.0000		
19	0.0000	0.0000	0.0000		
20	0.0000	0.0000	0.0000		

TOTAL SEDIMENT TRANSPORT = 2.3342 CFS

SEDIMENT TRANSPORT BY SIZE FRACTION IN CFS

1	2	3	4	5	5	7	8	9	10
.4435E-07	.8088E-05	.5107E-03	.1216E-01	.1222E+00	.5131E+00	.8829E+00	.6213E+00	.1659E+00	.1616E-01

SEDIMENT TRANSPORT BY SURFECTION AND SIZE FRACTION, IN CFS

[illegible]

[illegible]

Pump Sampler Example Output (cont.)

OVER ALL FLOW FREQUENCY WEIGHTED, RATIO OF SAMPLED SEDIMENT CONCENTRATION TO AVERAGE,
BY SUBSECTION AND SIZE FRACTION

SUB SECTION	1	2	3	4	5	6	7	8	9	10
1	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
2	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
3	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
4	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
5	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
6	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
7	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
8	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
9	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
10	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
11	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
12	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
13	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
14	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
15	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
16	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
17	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
18	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
19	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000
20	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000	0.00000

OVER ALL FLOW FREQUENCY WEIGHTED, RATIO OF SAMPLED CONCENTRATION TO AVERAGE,
BY SUBSECTION

SUBSECTION	RATIO
1	0.00000
2	0.00000
3	0.00000
4	0.00000
5	0.00000
6	0.00000
7	0.00000
8	0.00000
9	0.00000
10	0.00000
11	0.00000
12	0.00000
13	0.00000
14	0.00000
15	0.00000
16	0.00000
17	0.00000
18	0.00000
19	0.00000
20	0.00000

PROGRAM APPLICATION

General

Field data adequate to evaluate the program are unavailable at this time, thus verification and calibration of the program are left to later studies. This section will outline how the program can be used until further data are available. The first and most important point to remember is that the program is best used as a comparative tool. That is, it should be used to determine which of two samplers performs better or where in a cross section to locate a sampler for optimum operation. Its use as a predictive tool is limited. It should not be used to predict the total sediment load of a stream or the concentration of a particular sample. The uncertainties in these calculations are too large and errors in sediment transport may exceed 100 percent. But since the model is physically based on the principles of hydraulics and sediment transport any errors should be conservative (they will be in all calculations). When any two calculations are compared, such as the difference or better yet the ratio of sediment transport between two size fractions, the errors should cancel each other out and the user will be able to make valid decisions.

The program can be applied in two basic manners, either design of sediment samplers or calculation of actual sediment load from measured data. The following sections describe each of these applications.

Sampler Design

The application of the program in the design of sediment samplers is straightforward. The user enters the stream parameters where the sampler will be used and trial sampler parameters. The stream parameters will include channel geometry, friction slope, bed material, flow

discharge and flow frequency. The sampler parameters will include sampler type and dimensions. The program will then calculate the performance of the trial sampler at each channel subsection. The sampler design can then be changed and the program rerun, and the two designs compared. The user can continue this process until the best possible design is found.

If the user wishes to design a sampler for use on several different streams the trial and error process should be continued where the stream parameters are varied along with the sampler dimensions. This will yield not only the best sampler design, but also where in each stream the sampler should be placed. Thus the "best" design may be placed at the thalweg for one stream and higher in the section for another.

Calculation of Actual Sediment Load from Measured Data

The program can be used to calculate the actual sediment load in the stream from the measured sediment samples. To do this the user first enters the stream and sampler parameters for the flows where samples were taken and runs the program. The user then uses the results for the subsection where the sampler was located and ignores the rest. To calculate the actual average stream concentration for size fraction the user divides by the ratio of sample sediment concentration to the average. As an example consider the example box sampler (page 25) run with a discharge of 250 cfs and a sampler located in the channel thalweg (subsections 10 or 11). If the sampled sediment concentration for particle size 5 (0.0023 ft) was 100 mg/l divide by the output result of 1.87238 to obtain the actual average concentration for that flow of 53 mg/l.

Until the program is calibrated and verified, care should be taken in this application.

CONCLUSIONS AND RECOMMENDATIONS

The program has been developed with generated data which are typical of sampling conditions experienced in National Forests. In its development, the program has been found to be numerically stable under all conditions. Once familiar with the program users should have little problem with its application and are encouraged to modify the code to suit their needs. It is difficult to make any definitive conclusions without field data, but four statements can be made. First any box sampler which samples a vertical slice of the flow is superior to a point sampler which only samples at most a few spots. A box sampler that can measure the bed load and at least a part of the suspended load will have sampled concentrations much closer to the actual for both total load and load by size fraction. Its ability to sample the bed load is very important when evaluating transport in steep streams. Second, there is no perfect spot to place a sampler in a cross section. In a non-uniform channel there is no spot where a sampler will have the same sampling performance for all size fractions or all flows. Each sampler and stream must be evaluated on an individual basis. Third, for a sampler located at the stream thalweg, a sediment sample will have a larger concentration than the total flow of the finest particle sizes and a smaller concentration than the total flow of the largest. This is due to the lateral and vertical distribution of sediment in the channel. Fourth and last, it is difficult to design a sampler to work equally well at high and low flows. Review of the example output will show that for a given sampler the best location (as measured by the ratio of sampled sediment concentration to average) will move from the channel

thalweg for low flows up to higher spots as flow increases. The box sampler works well at all flows but the point sampler is very sensitive to flow volume. A good design for high flow will leave most of the sample ports dry at low flow.

The following two recommendations are made to improve the applicability of this program. First, field and laboratory data should be obtained from the literature or from new studies to calibrate and verify the model. Insuring that the flow resistance and sediment transport components of the model are adequate for the cases of interest should be of particular concern. Second, laboratory studies should be carried out to define the hydraulics and entrance effects of the specific sediment samplers used.

APPENDIX A

SAMPLER PROGRAM THEORY

GENERAL

This appendix presents the theory used in program SAMPLER. There are four basic steps in the calculation of the sediment: 1) flow hydraulics, 2) sediment properties, 3) sediment transport, and 4) sampling. The following sections describes the methods and assumptions used in each of the steps and explains some of the more important assumptions. An effort has been made to point out where improvements can be made or different approaches could be used.

FLOW HYDRAULICS

General

The flow hydraulics are computed assuming one-dimensional, uniform, steady flow. This implies the following assumptions: 1) the hydraulic characteristics of flow remain constant for the time interval under consideration; 2) the flow streamlines are practically parallel, i.e., a hydrostatic pressure distribution prevails over the channel section, and 3) the secondary flow (lateral or cross-stream) is negligible when compared to the longitudinal flow. In the program, the hydraulic properties are calculated for each discharge using a digitized channel geometry, and Manning's equation for flow resistance.

Manning's Equation

Many equations have been developed to determine channel resistance. The Manning equation is the most well known and used, and is thus used here. Another method such as described by Bathurst, Li and Simons (1979) is recommended if the stream of interest has a cobble and boulder

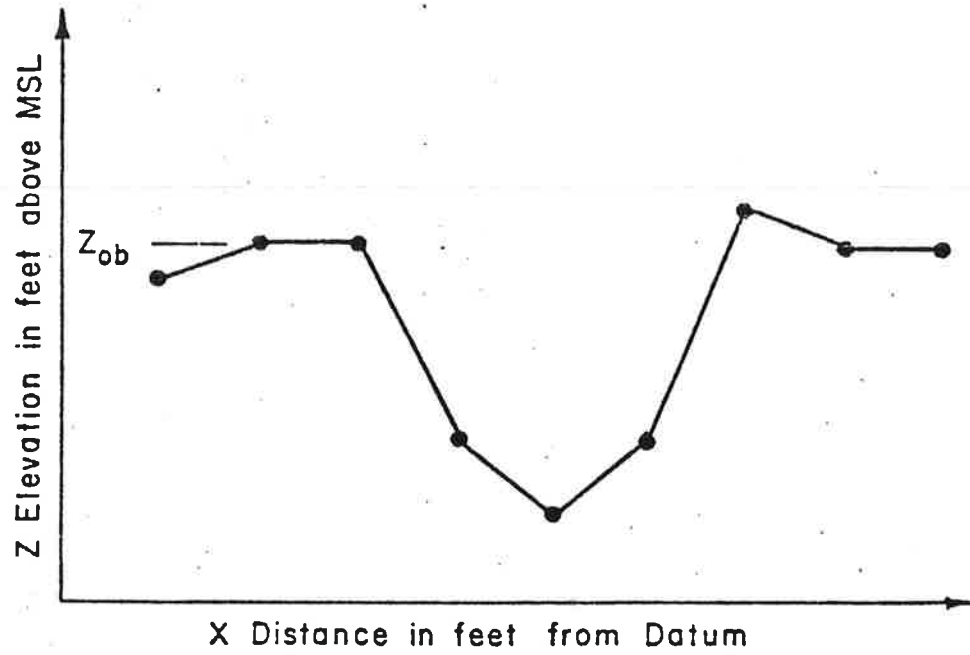


Figure A.1. Typical channel cross section.

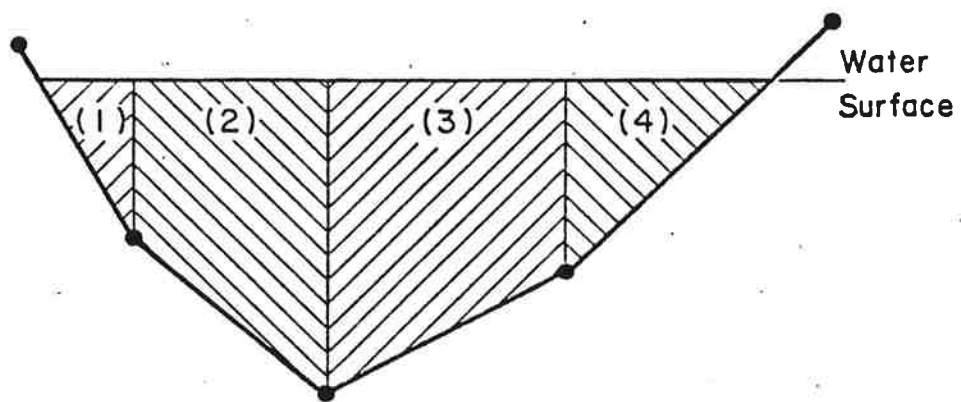


Figure A.2. Incremental areas in a cross section.

bed. The Manning equation is an empirical, uniform, steady flow equation. Errors are made when it is applied to nonprismatic natural channels with unsteady flow. It is believed that these errors are small when compared with those ordinarily occurring in the use of an uniform flow formula in natural channels (Chow, 1959). Likewise, it is believed there is no need to differentiate between total flow of water and sediment and just water flow.

The Manning equation is:

$$Q = \frac{1.486}{n} A R^{2/3} S_f^{1/2} \quad (\text{English units}) \quad (\text{A.1})$$

where Q = water discharge

A = water flow area

R = hydraulic radius

n = Manning channel roughness factor

Equation A.1 can be directly solved only for steady, uniform flow in channels with regular (triangle, rectangle, trapezoid, etc.) cross sections. In all other cases it must be solved in an iterative process.

Computation of Hydraulic Properties

The solution of Equation A.1 requires determination of channel hydraulic properties, area, and hydraulic radius at various depths. In these computations the hydraulic properties are computed with relationships developed using digitized channel geometry.

Channel cross sections are defined by (x,z) sets of coordinates. Figure A.1 shows a typical cross section. To allow for different Manning's n -values across the section each point is given its own n -value.

Area of flow for a given water surface elevation is computed by summing incremental areas between consecutive coordinates of the cross section. Figure A.2 illustrates this technique. Total area of flow is the summation of the increment areas, a_i :

$$A = \sum_{i=1}^N a_i \quad (\text{A.2})$$

where N is the total number of cross section incremental areas. Incremental areas are computed by:

$$a_i = X_b D_a \quad (\text{A.3})$$

where X_b is defined in Figure A.3.

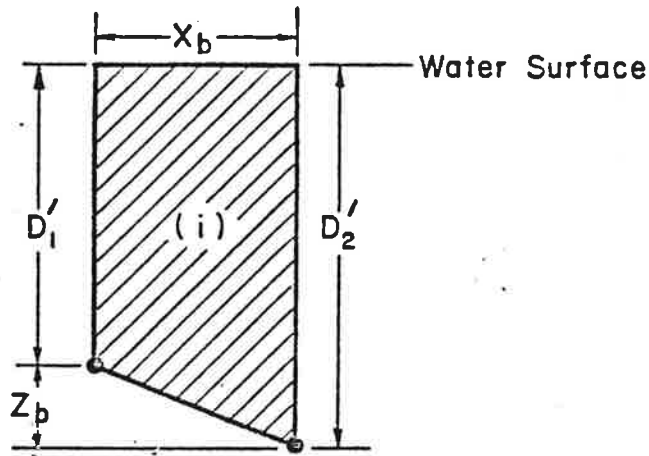


Figure A.3. Subsection area.

D_a is defined as:

$$D_a = \frac{1}{2} (D'_1 + D'_2) \quad (\text{A.4})$$

where D'_1 and D'_2 are defined in Figure A.3. If the water surface intercepts the cross section between coordinate points as shown by

increment 4 in Figure A.2, straight line interpolation between the points is used to compute the triangular area.

The wetted perimeter p_i is the length of the cross section below the water surface and is computed in increments by:

$$p_i = \sqrt{x_b^2 + z_b^2} \quad (A.5)$$

where z_b is defined in Figure A.3.

The incremental hydraulic radius, r_i , is calculated by:

$$r_i = \frac{a_i}{p_i} \quad (A.6)$$

The flow velocity for each increment is computed by the Manning's equation:

$$v_i = \frac{1.486}{n_{a_i}} r_i^{2/3} S_f^{1/2} \quad (A.7)$$

where n_{a_i} is the average Manning's n value at the two coordinate points which define the increment.

The incremental flow discharge is computed by continuity:

$$q_i = v_i a_i \quad (A.8)$$

where q_i is the incremental flow area and the total discharge is:

$$Q = \sum_{i=1}^N q_i \quad (A.9)$$

Solution for Depth of Flow

As stated previously, solution of the Manning's equation for nonregular cross requires an iterative solution method. In the program for each discharge an initial water surface elevation is assumed at the top of the section. For the trial elevation and the given friction

slope the Manning equation is used to compute a trial discharge. If the trial and actual discharges are equal the correct water surface and flow hydraulics has been found. If the two discharges are not equal the program reduces the assumed depth and tries again. These steps are repeated until the correct water elevation and flow hydraulics are found.

SEDIMENT PROPERTIES

General

Two of the sediment properties are of concern here, the particle fall velocity and the size distribution. In the program the particle fall velocity is calculated by Rubey's (1933) equation while the particle size distribution is generated with a log-normal fit.

Fall Velocity

Rubey's equation is used to calculate the particle fall velocity.

Rubey's equation is:

$$w_i = \frac{(2/3 g (S_s - 1) d_i^3 + 36v^2)^{1/2} - 6v}{d_i} \quad (A.10)$$

where g = acceleration of gravity

S_s = specific weight of the sediment

d_i = sediment size

v = kinematic viscosity of the water.

The specific weight is assumed to be 2.65. The kinematic viscosity is a function of water temperature and is assumed to be 1.1×10^{-5} . Since the fall velocity is a function of particle size the equation is applied to each size of interest.

Sediment Size Distribution

In many cases an accurate sediment size distribution is not available for the site of a sampler or the performance of a sampler need be determined without knowing where it may be placed. Because of these reasons, the program has been designed to calculate reasonable sediment distributions with minimum data. The program requires only the sediment sizes of interest and the median sediment size.

Figure A.4 shows a typical sediment size distribution. The median sediment size, d_{50} is defined as the particle size for which 50 percent of all particles are small than. For the program the user must define the sediment sizes of interest by inputting the upper and lower particle sizes of each range. The program calculates the geometric mean, d_{g_i} for each sediment size range and uses that value for the sediment transport.

$$d_{g_i} = (d_j d_{j+1})^{1/2} \quad (A.11)$$

where d_j and d_{j+1} are the minimum and maximum sizes in the size range i . The program calculates the percentage of sediment in each size range using a log-normal distribution. The standard deviation, σ_g of the log-normal distribution is defined as

$$\sigma_g = \left(\frac{d_{84.1}}{d_{15.9}} \right)^{1/2} \quad (A.12)$$

Where $d_{84.1}$ and $d_{15.9}$ are the sizes of the 84.1 and 15.9 percent finer sizes. The program assumes $\sigma_g = 2.0$. Thus with the d_{50} and σ_g the program can calculate the percentage of material in each size range. In Figure A.4 the two distributions have the same $d_{84.1}$, $d_{15.9}$, and σ_g .

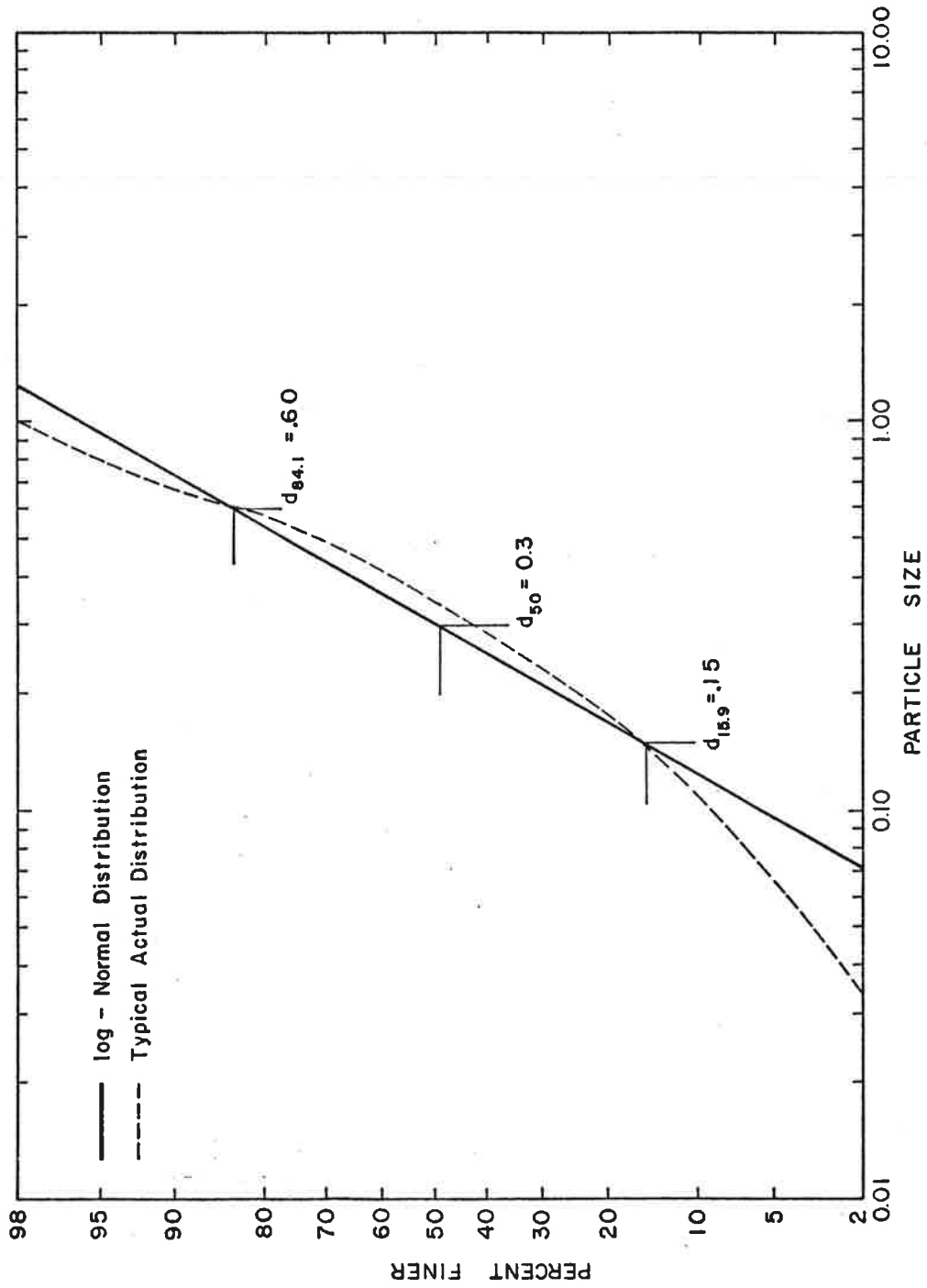


Figure A.4. Sediment size distribution.

SEDIMENT TRANSPORT

General

No theoretically complete equation for sediment transport has been developed. The reason for this lack is two-fold. First, the processes by which flowing water imparts energy to sediment particles is poorly quantified. Second, not all sediment particles are created equal. They vary in size, shape, and density. Some particles in the clay sizes are influenced strongly by ionic forces, some are not. In a given flow some particles travel entirely in suspension, some entirely by rolling along the bed, and others by a combination. Some particles can be supply limited at a given moment while at the same time other size particles are transport limited. Considering the complexity and variability of alluvial systems a complete, theoretical solution to the sediment transport equation may evade us for a long time. For the present, the engineer can select from numerous empirical equations. Simons and Sentürk (1977) present a review of the most widely known equations. In the program the Meyer-Peter and Müller bed load formula is used along with Lane and Kalinskes suspended sediment distribution.

Bed Load

The first step in the analytical calculation of sediment transport is the determination of the bed load. The bed load is defined as the transport rate of those particles which move by rolling or sliding along the bed. The Meyer-Peter and Müller (1948) formula has been found to give reasonable results in steep sand and gravel streams and is used here.

The formula developed by Meyer-Peter and Müller (MPM) is an excess shear stress type formula of the form normally classified as a DuBoy's type. The MPM formula can be written

$$q_{bv} = \frac{8 (g/\gamma)^{1/2}}{\gamma'_s} (\tau_o - \tau_c)^{3/2} \quad (A.13)$$

where: q_{bv} = volumetric bed load transport per unit width of channel,
 τ_o = flow shear stress on the bed,
 τ_c = sediment particle critical shear stress for initiation of motion,
 γ = specific weight of water, and,
 γ'_s = specific weight of submerged sediment.

Equation A.13 has units of length squared per time. The dimensionless coefficient with a value of eight is the result of correcting the equation to match experimental results. The flow shear stress can be evaluated a number of ways but in applications of the MPM it is customary to calculate τ_o by the relation,

$$\tau_o = \gamma DS \quad (A.14)$$

Equation A.14 is a measure of the total tractive forces which the bed exerts on the flow, and can be obtained from a control volume analysis of a wide channel with uniform flow. The critical shear stress, τ_c is that minimum flow shear stress at which a particular size sediment particle will be transported by the flow. It is a function of the sediment particle size, shape and density. Meyer-Peter and Müller developed the relationship:

$$\tau_c = 0.047 \gamma'_s d \quad (A.15)$$

The dimensionless coefficient 0.047 in Equation A.15 was obtained from experiments. This relationship is a simplification of a very complex process. Shields (1936) developed a diagram developed from experiments. The original diagram has been improved by Gessler (1965) and is the most

widely used method to determine τ_c . The Shields diagram is not used here for two reasons. First, it is a graphical procedure that does not lend itself easily to numerical models and second, the difference between the MPM approach and Shields' approach does not exceed 22 percent when considering a completely rough boundary (Simons and Sentürk, 1977). This difference is considered small.

Since Equation A.15 is a function of the sediment size and there is normally a range of sediment sizes in the bed, the equation is applied to each particle size fraction and the result multiplied by the percentage by weight that the size fraction occurs in the bed, or

$$q_{bv} = \sum_{i=1}^n p_i \frac{\gamma(g/\gamma)^{1/2}}{\gamma_s'} (\tau_o - \tau_{c_i})^{3/2} \quad (A.16)$$

Where: i = particle size fraction

n = total number of size fraction

p_i = percentage of fraction i of the total bed.

In the application of Equation A.16 if the critical shear stress exceeds the flow shear stress, the transport is set equal to zero. Currently the program does not consider hiding effects where the larger size fractions hide the smaller sizes from the flow. If these effects are found to be significant, methods such as those proposed by Proffitt and Sutherland (1983) can be used or a single dominant shear stress based on d_{50} when all particles initiate motion as proposed by Parker, Klingaman and McLean (1982) used.

SUSPENDED BED LOAD

The suspended bed load is defined as the transport in suspension of those size fractions that occur in the bed. Suspended bed load does

not include the wash load. Wash load is the sediment which the supply from the watershed is less than the transport capacity of the stream. Since the wash is supply limited it cannot be calculated by the flow hydraulics.

Lane and Kalinske's (1941) sediment distribution is used to evaluate the suspended sediment concentrations. This program was initially developed with the Einstein (1950) distribution but serious difficulties arise in the application due to its use. A weakness of the Einstein distribution is that in its development it implicitly assumed that the sediment concentration at the water surface is zero. This assumption causes no practical problems for large particles, but for fine sands and silts in steep streams it produces an unreasonable distribution near the water surface. In its normal application where only the total transport is of concern zero surface concentration assumption causes no real problems, since the distribution is reasonable at depths greater than one-tenth the total depth of flow. But in this application where it is of great importance what concentration a sampler would measure at all depths for all particles, it causes unacceptable errors. The errors are both conceptual and numerical. Integrating the function near the surface is difficult.

Lane and Kalinske's distribution does not suffer from the zero surface concentration assumption and is a much simpler function. Figure A.5 shows a comparison between the Einstein and Lane and Kalinske's distributions. The distribution is

$$C_y = C_a \exp \left[\frac{-6W}{u_*' \beta K D} (y - a) \right] \quad (A.17)$$

where: u_*' = grain friction flow shear velocity

C_y = sediment concentration at a distance y above the bed

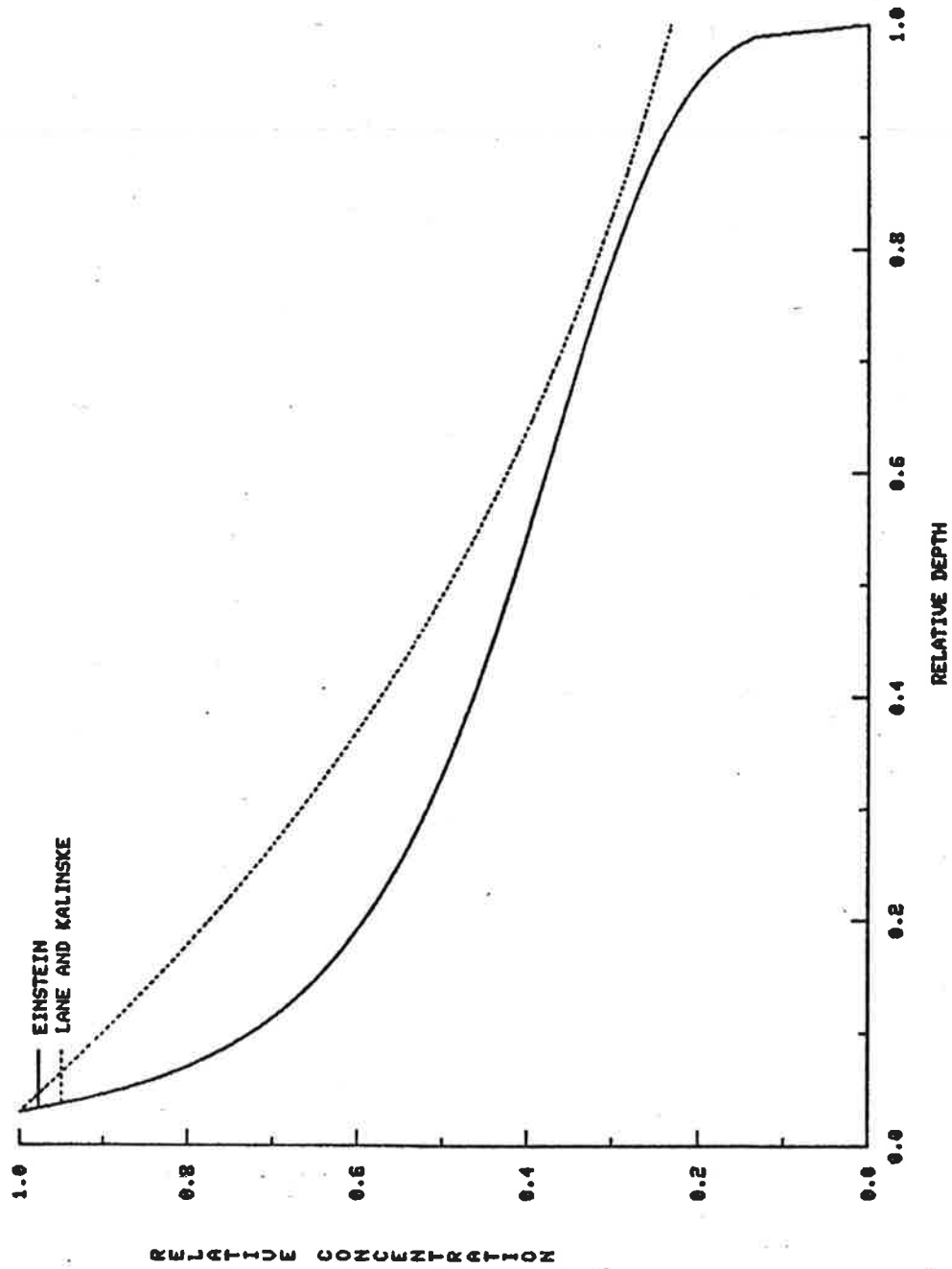


Figure A.5. Comparison between Einstein and Lane and Kalinske's sediment concentrations.

C_a = reference concentration at a distance a above the bed

κ = von Karman constant

y, a = distances above the bed

β = correction coefficient.

The value six in Equation A.17 is obtained from an integration and is a pure number. As in most studies κ is defined equal to 0.40 and β is assumed equal to 1.0. Equation A.17 can then be rewritten as

$$C_y = C_a \exp \left[\frac{-15w}{u_*' D} (y - a) \right] \quad (\text{A.18})$$

The grain friction flow shear velocity u_*' in this application is evaluated by a form of the Darcy-Weisbach formula

$$u_*' = \frac{f}{8} \rho V^2 \quad (\text{A.19})$$

where f = friction factor

ρ = water density

V = flow velocity.

The water density is assumed constant with $\rho = 1.938$ (English units).

The value eight in Equation A.19 is a pure number.

To determine the total sediment transport between two depths y_1 and y_2 Equation A.18 is integrated with respect to y and evaluated between depths y_1 and y_2 to yield

$$q_{sv} = \int_{y_1}^{y_2} C_y dy = C_a \frac{D}{z} e^{\frac{-15w}{u_*'} \frac{(y_2 - a)}{D}} - e^{\frac{-15w}{u_*'} \frac{(y_1 - a)}{D}} \quad (\text{A.20})$$

where q_s is the volume rate of suspended bed material between depths y_1 and y_2 .

All variables in Equation A.20 are known except the reference depth a and the corresponding concentration C_a . Einstein (1954) used the bed load to calculate the reference concentration

$$C_a = \frac{q_{bv_i}}{a_i} \quad (A.21)$$

where a is twice the particle size or

$$a = 2 \cdot d_i \quad (A.22)$$

As in the bed load calculations Equation A.20 is summed over each particle size; or

$$q_{sv} = \sum_{i=1}^n p_i \frac{q_{bv_i}}{a_i} \frac{Ca_i}{(-15w_i \frac{a_i}{d})} \frac{d}{z} e^{\frac{-15w_i}{u_*} \frac{y_2}{d}} - e^{\frac{-15w_i}{u_*} \frac{y_1}{d}} \quad (A.23)$$

For small particles the value $a_i = 2 d_i$ will give unreasonably high reference concentrations, therefore a lower limit is set on a_i such that $a_i \geq 0.01$ feet.

SAMPLING

General

Calculation of the theoretical sample concentration requires knowledge of the water velocity profile, the sediment concentration profile, the sampler geometry and the hydraulic performance of the sample. This study is limited to consideration of the first three items only. The hydraulic performance of a sampler cannot be adequately predicted by analytical procedures. While some theoretical evaluations are possible, it was decided to limit the scope of this contract and to leave more detailed analysis to later studies.

In this report two types of samples are considered. The first is a "box" sampler. The box sampler collects a vertical section segment of the flow from the bed to the top of the sampler or the water surface,

whichever is less. This sampler replicates the intake geometry of converging section samplers. The second sampler is a multiple intake "point" sampler. This sampler has multiple ports located at various points above the bed. This sampler replicates the geometry of standard pump samples.

Since the calculations are so different for these two types of samplers, two completely different subroutines were written for the program. The user selects which routine he wishes to use.

Box Sampler

The box sampler routine assumes a vertical slice of water and sediment is taken. The sampler is assumed to take the slice from the bottom to the top of the sampler or the water surface, whichever is less. The sampler is assumed to measure the entire bed load and that portion of the suspended load into which it projects. The sampler is assumed to have constant intake dimensions of width and height. Figure A.6 shows the box sampler. Since the program does not presently consider the sampler hydraulics, this sampler geometry is applicable to any sampler which takes a vertical section.

The box sampler takes a sample from the bottom to the sampler top including all the bed load. When Equation A.16 is used to calculate the bed load, the measured sediment load, q_m , is calculated by:

$$q_m = q_{bv} + q_{svm} \quad (A.24)$$

where q_{svm} is the suspended load sediment between a_i and the sampler height, H . From Equation A.23:

$$q_{svm} = \sum_{i=1}^n P_i \frac{q_{bv_i} H}{a_i} \int_a^{DU} Ca_i \frac{DU}{-15w} \left(e^{\frac{-15w_i}{U^*} \left(\frac{H-q_i}{D} \right)} - 1 \right) \quad (A.25)$$

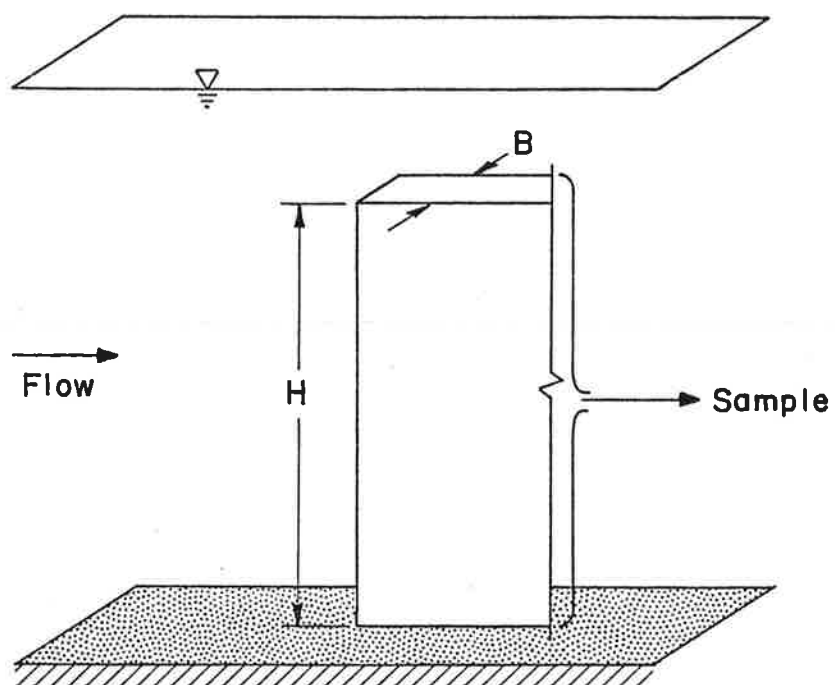


Figure A.6. Box sampler.

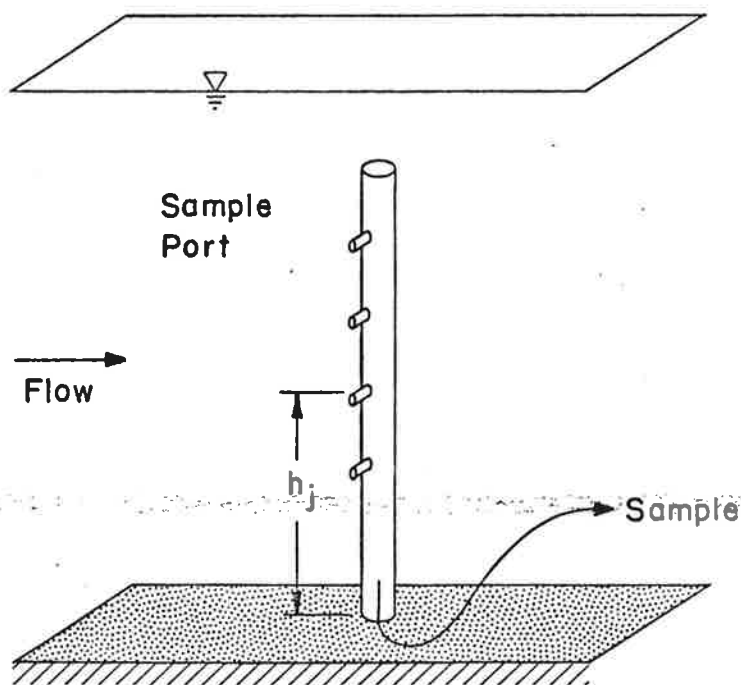


Figure A.7. Point sampler.

Sediment particles larger than the sampler width cannot be caught, likewise particle sizes that are a significant percentage of the sampler opening will many times be rejected. An empirical entrance effect has been defined for the model. The entrance function R_e is:

$$\begin{aligned} R_{e_i} &= 1.0 & d_i &\leq W/2 \\ R_{e_i} &= 2 - \frac{d_i}{W/2} & \frac{W}{2} < d_i \leq W \\ R_{e_i} &= 0 & d_i &> W \end{aligned} \quad (A.26)$$

The entrance effect is multiplied with Equation A.25:

$$q_{m_i} = R_{e_i} (q_{bv_i} + q_s v_{m_i}) \quad (A.27)$$

The sample sediment concentration is calculated using the value of water the sampler collects, q_w , or

$$q_w = v H W \quad (A.28)$$

where v is the subsection velocity and is defined by Equation A.7 and W is the sampler width. If the depth of flow is less than H it is used. The sampled concentration, C_{s_i} is thus:

$$C_{s_i} = \frac{q_{m_i}}{q_w} \quad (A.29)$$

Point sampler

The second sampler considered is a point sampler. This sampler has multiple intake ports located at various points above the bed. Most pump samples can be simulated with this design. The sampler is assumed to measure the suspended load at each port and then combines the samples

from each port. It is assumed none of the bed load is sampled, and sampler has constant dimensions. Figure A.7 shows the sampler.

The concentration at each point is calculated by Equations A.19, A.21 and A.22. The concentration of sediment size i at port j is

$$C_{y_{ij}} = \frac{q_{bv_i}}{a_i} \exp \left[\frac{-15w_i}{U_*'D} (h_j - a_i) \right] \quad (A.30)$$

where h_j is the height of the port. The flow velocity at the port is calculated with a logarithmic distribution developed by Brooks (1963),

$$v_j = v + \frac{U_*'}{K} \ln \left(\frac{hp}{D} + 1 \right) \quad (A.31)$$

The mean flow velocity v , is calculated by Equation A.7 and U_*' by Equation A.19.

The volume of water per time in the sample q_{wj} at a port is obtained by multiplying the flow velocity by the port area A_p .

$$q_{wj} = v_j \cdot A_p \quad (A.32)$$

The volume of sediment per time in the sample at a port is

$$q_{sv_{ij}} = q_{wj} C_{y_{ij}} R_{e_i}$$

where R_{e_i} , the entrance effect, is again calculated by Equation A.26.

The sampled concentration of the combined sample is then

$$C_{s_i} = \frac{\sum_{j=1}^W q_{sv_{ij}}}{\sum_{j=1}^W q_{wj}} \quad (A.33)$$

APPENDIX B
PROGRAM LISTING

PROGRAM SAMPLER	83	SAM 0010
1(OUTPUT,TAPE5,TAPE6=OUTPUT)		SAM 0020
C		SAM 0030
C THIS IS A PROGRAM FOR SEDIMENT SAMPLER EVALUATION.		SAM 0040
C THE PROGRAM WAS DEVELOPED BY G.O. BROWN AND R.M. LI,		SAM 0050
C AT COLORADO STATE UNIVERSITY.		SAM 0060
C		SAM 0070
COMMON FO , S , ND , NSIZ ,		SAM 0080
1 Q(10) , FREQ(10) , QSJ(10) , X(30) ,		SAM 0090
2 Z(30) , F(30) , Q1(30) , Y1(30) ,		SAM 0100
3 XB(30) , V1(30) , XN(30) , DMB(10) ,		SAM 0110
4 DBD(11) , FVB(10) , QSD(10) , QSDSUM(10) ,		SAM 0120
5 RD(10) , RDSUM(30,10) , PD(30,10) , QSV(30,10) ,		SAM 0130
6 QBV(30,10) , QTV(30,10) , EFS(30,10) , RATIO(30,10) ,		SAM 0140
7 RATIOK(30) , QSK(30) , RKSUM(30) , RK(30) ,		SAM 0150
8 D(30)		SAM 0160
C		SAM 0170
DIMENSION TITLE(12)		SAM 0180
DATA SNU/.000011/		SAM 0190
C		SAM 0200
C DEFINE DEVICE NUMBERS		SAM 0210
C		SAM 0220
C		SAM 0230
C INPUT DATA		SAM 0240
C		SAM 0250
C IR = SAMPLER NUMBER		SAM 0260
C ND = NUMBER OF POINTS IN THE CROSS SECTION		SAM 0270
C NQ = NUMBER OF DISCHARGES		SAM 0280
C NSIZ = NUMBER OF PARTICLE SIZE FRACTIONS		SAM 0290
C S = SLOPE		SAM 0300
C FO = GRAIN ROUGHNESS		SAM 0310
C Q = DISCHARGE		SAM 0320
C FREQ = FREQUENCY OF DISCHARGE		SAM 0330
C X = CROSS SECTION POINT HORIZONTAL DISTANCE		SAM 0340
C Z = CROSS SECTION POINT VERTICAL ELEVATION		SAM 0350
C F = CROSS SECTION POINT MANNING'S N VALUE		SAM 0360
C D = CROSS SECTION POINT MEDIUM SEDIMENT SIZE		SAM 0370
C DBD = PARTICLE SIZE LIMITS		SAM 0380
C		SAM 0390
C READ IN DATA		SAM 0400
C		SAM 0410
READ (5,370) (TITLE(M),M = 1,12)		SAM 0420
READ (5,210) IR,ND,NQ,NSIZ,S,FO		SAM 0430
READ (5,220) (Q(J),FREQ(J),J = 1,NQ)		SAM 0440
READ (5,230) (X(K),Z(K),F(K),D(K),K = 1,ND)		SAM 0450
NSP1 = NSIZ + 1		SAM 0460
READ (5,240) (DBD(I),I = 1,NSP1)		SAM 0470
WRITE (6,380)		SAM 0480
WRITE (6,390) (TITLE(M),M = 1,12)		SAM 0490
WRITE (6,250) IR		SAM 0500
WRITE (6,400) ND,NQ,NSIZ,S,FO		SAM 0510
C		SAM 0520
C FIND HIGH AND LOW ELEVATIONS IN SECTION		SAM 0530
C		SAM 0540
ZMIN0 = 100000.		SAM 0550
ZMAX0 = - 10000.		SAM 0560
DO 100 K = 1,ND		SAM 0570

IF (Z(K).LT.ZMIN0) ZMIN0 = Z(K)	SAM 0580
IF (Z(K).GT.ZMAX0) ZMAX0 = Z(K)	SAM 0590
100 CONTINUE	SAM 0600
WRITE (6,410)	SAM 0610
WRITE (6,420) (K,X(K),Z(K),F(K),D(K),K = 1,ND)	SAM 0620
C	SAM 0630
C CALL SUBROUTINE PROPAR TO GENERATE THE SIZE DISTRIBUTION	SAM 0640
C	SAM 0650
CALL PROPAR	SAM 0660
WRITE (6,430)	SAM 0670
WRITE (6,440)	SAM 0680
WRITE (6,450)	SAM 0690
WRITE (6,310) (DMB(I),I = 1,NSIZ)	SAM 0700
NDM1 = ND - 1	SAM 0710
C	SAM 0720
C CALCULATE FALL VELOCITY	SAM 0730
C	SAM 0740
DO 110 I = 1,NSIZ	SAM 0750
FVB(I) = (SQRT(35.417 * DMB(I) * * 3 + 36.0 * SNU * * 2) - 6.	SAM 0760
1 * SNU)/DMB(I)	SAM 0770
110 CONTINUE	SAM 0780
WRITE (6,460)	SAM 0790
WRITE (6,310) (FVB(I),I = 1,NSIZ)	SAM 0800
WRITE (6,470)	SAM 0810
WRITE (6,330) (K,(PD(K,I),I = 1,NSIZ),K = 1,NDM1)	SAM 0820
C	SAM 0830
C INITIALIZATION	SAM 0840
C	SAM 0850
QSJSUM = 0.	SAM 0860
DO 130 K = 1,NDM1	SAM 0870
RKSUM(K) = 0.	SAM 0880
DO 120 I = 1,NSIZ	SAM 0890
QSDSUM(I) = 0.	SAM 0900
RDSUM(K,I) = 0.	SAM 0910
120 CONTINUE	SAM 0920
130 CONTINUE	SAM 0930
C	SAM 0940
C ITERATE OVER EACH DISCHARGE	SAM 0950
C	SAM 0960
DO 170 J = 1,NQ	SAM 0970
WRITE (6,280) (J,Q(J),FREQ(J))	SAM 0980
ZMAX = ZMAX0	SAM 0990
ZMIN = ZMIN0	SAM 1000
C	SAM 1010
C CALL SUBROUTINE SUBQ TO DIVIDE THE TOTAL DISCHARGE INTO	SAM 1020
C EACH SUBSECTION	SAM 1030
C	SAM 1040
CALL SUBQ (J,ZMAX,ZMIN)	SAM 1050
WRITE (6,480)	SAM 1060
WRITE (6,290) (K,Q1(K),V1(K),Y1(K),K = 1,NDM1)	SAM 1070
C	SAM 1080
C CALCULATE THE SEDIMENT TRANSPORT	SAM 1090
C	SAM 1100
CALL TRANSP (J)	SAM 1110
WRITE (6,300) QSJ(J)	SAM 1120

WRITE (6,490)	SAM 1130
WRITE (6,310) (QSD(I),I = 1,NSIZ)	SAM 1140
WRITE (6,320)	SAM 1150
WRITE (6,440)	SAM 1160
WRITE (6,330) (K,(QTV(K,I),I = 1,NSIZ),K = 1,NDM1)	SAM 1170
C	SAM 1180
C CALCULATE WHAT THE SAMPLER CATCHES	SAM 1190
C	SAM 1200
IF (IR.EQ.1) CALL BOX (J)	SAM 1210
IF (IR.EQ.2) CALL PUMP (J)	SAM 1220
WRITE (6,340)	SAM 1230
WRITE (6,440)	SAM 1240
WRITE (6,330) (K,(EFS(K,I),I = 1,NSIZ),K = 1,NDM1)	SAM 1250
WRITE (6,350)	SAM 1260
WRITE (6,440)	SAM 1270
WRITE (6,330) (K,(RATIO(K,I),I = 1,NSIZ),K = 1,NDM1)	SAM 1280
DO 150 K = 1,NDM1	SAM 1290
RKSUM(K) = RKSUM(K) + RATIO(K) * QSJ(J) * FREQ(J)	SAM 1300
DO 140 I = 1,NSIZ	SAM 1310
RDSUM(K,I) = RDSUM(K,I) + RATIO(K,I) * QSD(I) * FREQ(J)	SAM 1320
140 CONTINUE	SAM 1330
150 CONTINUE	SAM 1340
QSJSUM = QSJSUM + QSJ(J) * FREQ(J)	SAM 1350
DO 160 I = 1,NSIZ	SAM 1360
QSDSUM(I) = QSDSUM(I) + QSD(I) * FREQ(J)	SAM 1370
160 CONTINUE	SAM 1380
170 CONTINUE	SAM 1390
C	SAM 1400
C CALCULATE THE OVER ALL CATCH RATIOS	SAM 1410
C	SAM 1420
WRITE (6,360)	SAM 1430
WRITE (6,440)	SAM 1440
DO 200 K = 1,NDM1	SAM 1450
RK(K) = RKSUM(K)/QSJSUM	SAM 1460
DO 190 I = 1,NSIZ	SAM 1470
IF (QSDSUM(I).EQ.0.0) GO TO 180	SAM 1480
RD(I) = RDSUM(K,I)/QSDSUM(I)	SAM 1490
GO TO 190	SAM 1500
180 RD(I) = - 1.0	SAM 1510
190 CONTINUE	SAM 1520
WRITE (6,330) (K,(RD(I),I = 1,NSIZ))	SAM 1530
200 CONTINUE	SAM 1540
WRITE (6,260)	SAM 1550
WRITE (6,270) (K,RK(K),K = 1,NDM1)	SAM 1560
C	SAM 1570
STOP	SAM 1580
C	SAM 1590
C	SAM 1600
210 FORMAT (4I5,2F10.4)	SAM 1610
220 FORMAT (2F10.2)	SAM 1620
230 FORMAT (4F10.4)	SAM 1630
240 FORMAT (F10.6)	SAM 1640
250 FORMAT (//,1X,16HSAMPLER NUMBER =,I5)	SAM 1650
260 FORMAT (//,1X,50HOVER ALL FLOW FREQUENCY WEIGHTED, RATIO OF SAMPLES	SAM 1660
1D,26H CONCENTRATION TO AVERAGE,,/,1X,13HBY SUBSECTION,/,1X,19HSUBS	SAM 1670

END

	SUBROUTINE SUBQ (J,ZMAX,ZMIN)	87	SUB 0010
C			SUB 0020
C	THIS SUBROUTINE DETERMINES THE DEPTH OF FLOW AND SUBSECTION		SUB 0030
C	DISCHARGE.		SUB 0040
C			SUB 0050
	COMMON FO , S , ND , NSIZ ,		SUB 0060
1	Q(10) , FREQ(10) , QSJ(10) , X(30) ,		SUB 0070
2	Z(30) , F(30) , Q1(30) , Y1(30) ,		SUB 0080
3	XB(30) , V1(30) , XN(30) , DMB(10) ,		SUB 0090
4	DBD(11) , FVB(10) , QSD(10) , QSDSUM(10) ,		SUB 0100
5	RD(10) , RDSUM(30,10) , PD(30,10) , QSV(30,10) ,		SUB 0110
6	QBV(30,10) , QTV(30,10) , EFS(30,10) , RATIO(30,10) ,		SUB 0120
7	RATIOK(30) , QSK(30) , RKSUM(30) , RK(30) ,		SUB 0130
8	D(30)		SUB 0140
	DATA NAP/ 4/		SUB 0150
	DATA TOL/ .01/		SUB 0160
C			SUB 0170
C	TRIAL AND ERROR FOR DEPTH OF FLOW OVER TOTAL SECTION HEIGHT		SUB 0180
C			SUB 0190
	IET = 1		SUB 0200
100	ZDIF = ZMAX - ZMIN		SUB 0210
	DWS = ZDIF/FLOAT(NAP)		SUB 0220
	WS = ZMIN		SUB 0230
	DO 110 N = 1,NAP		SUB 0240
	WS = WS + DWS		SUB 0250
	CALL GEOM (WS,SUMQ1)		SUB 0260
	DQ = ABS(Q(J) - SUMQ1)		SUB 0270
	EPS = TOL * Q(J)		SUB 0280
C			SUB 0290
C	TEST FOR CONVERGENCE		SUB 0300
C			SUB 0310
	IF (DQ.LE.EPS) RETURN		SUB 0320
	IF (SUMQ1.GT.Q(J)) GO TO 120		SUB 0330
110	CONTINUE		SUB 0340
120	ZMAX = WS		SUB 0350
	ZMIN = WS - DWS		SUB 0360
	IET = IET + 1		SUB 0370
	IF (IET.LE.10) GO TO 100		SUB 0380
	WRITE (6,130)		SUB 0390
C			SUB 0400
	RETURN		SUB 0410
C			SUB 0420
130	FORMAT (1X, 38HDOES NOT CONVERGE WITHIN 10 ITERATIONS)		SUB 0430
	END		SUB 0440

```

SUBROUTINE GEOM (WS,SUMQ1)
C
C THIS SUBROUTINE CALCULATES THE EXACT HYDRAULIC PROPERTIES OF
C A CROSS SECTION, ONCE GIVEN THE CHANNEL GEOMETRY AND THE
C WATER SURFACE ELEVATION.
C
COMMON FO , S , ND , NSIZ ,
1 Q(10) , FREQ(10) , QSJ(10) , X(30) ,
2 Z(30) , F(30) , Q1(30) , Y1(30) ,
3 XB(30) , V1(30) , XN(30) , DMB(10) ,
4 DBD(11) , FVB(10) , QSD(10) , QSDSUM(10) ,
5 RD(10) , RDSUM(30,10), PD(30,10) , QSV(30,10) ,
6 QBV(30,10) , QTV(30,10) , EFS(30,10) , RATIO(30,10),
7 RATIOK(30) , QSK(30) , RKSUM(30) , RK(30) ,
8 D(30)

SUMQ1 = 0.
DO 140 K = 2,ND
  KM1 = K - 1
  Y1(KM1) = 0.
  V1(KM1) = 0.
  Q1(KM1) = 0.
  XB(KM1) = X(K) - X(KM1)
  IF (XB(KM1).EQ.0.) XB(KM1) = 1.0E - 6
  FM = 0.5 * (F(KM1) + F(K))
  XN(KM1) = FM
  IF (Z(K).GE.WS) GO TO 110
  IF (Z(KM1).GE.WS) GO TO 100
  DA = WS - 0.5 * (Z(KM1) + Z(K))
  Y1(KM1) = DA
  A = XB(KM1) * DA
  ZB = ABS(Z(K) - Z(KM1))
  P = SQRT(XB(KM1) * XB(KM1) + ZB * ZB)
  GO TO 130
100 ZB = WS - Z(K)
  XB(KM1) = XB(KM1) * ZB/(Z(KM1) - Z(K))
  IF (XB(KM1).EQ.0.) XB(KM1) = 1.0E - 6
  GO TO 120
110 IF (Z(KM1).GE.WS) GO TO 140
  ZB = WS - Z(KM1)
  XB(KM1) = XB(KM1) * ZB/(Z(K) - Z(KM1))
  IF (XB(KM1).EQ.0.) XB(KM1) = 1.0E - 6
120 A = 0.5 * XB(KM1) * ZB
  P = SQRT(XB(KM1) * XB(KM1) + ZB * ZB)
  DA = 0.5 * ZB
  Y1(KM1) = DA
130 CONTINUE

C
C CALCULATE TRIAL DISCHARGE USING MANNINGS EQUATION
C
R = A/P
V1(KM1) = 1.486/FM * S * * 0.5 * R * * (2./3.)
Q1(KM1) = V1(KM1) * A
SUMQ1 = SUMQ1 + Q1(KM1)
140 CONTINUE

```

GEO 0010
 GEO 0020
 GEO 0030
 GEO 0040
 GEO 0050
 GEO 0060
 GEO 0070
 GEO 0080
 GEO 0090
 GEO 0100
 GEO 0110
 GEO 0120
 GEO 0130
 GEO 0140
 GEO 0150
 GEO 0160
 GEO 0170
 GEO 0180
 GEO 0190
 GEO 0200
 GEO 0210
 GEO 0220
 GEO 0230
 GEO 0240
 GEO 0250
 GEO 0260
 GEO 0270
 GEO 0280
 GEO 0290
 GEO 0300
 GEO 0310
 GEO 0320
 GEO 0330
 GEO 0340
 GEO 0350
 GEO 0360
 GEO 0370
 GEO 0380
 GEO 0390
 GEO 0400
 GEO 0410
 GEO 0420
 GEO 0430
 GEO 0440
 GEO 0450
 GEO 0460
 GEO 0470
 GEO 0480
 GEO 0490
 GEO 0500
 GEO 0510
 GEO 0520
 GEO 0530
 GEO 0540
 GEO 0550

C	RETURN	GEO 0560
	END	GEO 0570
	SUBROUTINE TRANSP (J)	GEO 0580
C		TRA 0010
C	THIS SUBROUTINE CALCULATES SEDIMENT TRANSPORT USING MEYER-	TRA 0020
C	PETER-MULLER'S BED LOAD FUNCTION AND LANETAND 'KALINSKEE'S	TRA 0030
C	SUSPENDED LOAD FUNCTION.	TRA 0040
C		TRA 0050
	COMMON FO , S , ND , NSIZ ,	TRA 0060
1	Q(10) , FREQ(10) , QSJ(10) , X(30) ,	TRA 0070
2	Z(30) , F(30) , Q1(30) , Y1(30) ,	TRA 0080
3	XB(30) , V1(30) , XN(30) , DMB(10) ,	TRA 0090
4	DBD(11) , FVB(10) , QSD(10) , QSDSUM(10) ,	TRA 0100
5	RD(10) , RDSUM(30,10) , PD(30,10) , QSV(30,10) ,	TRA 0110
6	QBV(30,10) , QTV(30,10) , EFS(30,10) , RATIO(30,10) ,	TRA 0120
7	RATIOK(30) , QSK(30) , RKSUM(30) , RK(30) ,	TRA 0130
8	D(30)	TRA 0140
C		TRA 0150
	NDM1 = ND - 1	TRA 0160
	QSJ(J) = 0.	TRA 0170
	DO 100 I = 1,NSIZ	TRA 0180
	QSD(I) = 0.	TRA 0190
100	CONTINUE	TRA 0200
	PN = 8.0 * SQRT(32.2/62.4)/(2.65 * 62.4 - 62.4)	TRA 0210
C		TRA 0220
C	ITERATE OVER EACH SIZE FRACTION.	TRA 0230
C		TRA 0240
	DO 140 K = 1,NDM1	TRA 0250
	QSK(K) = 0.	TRA 0260
	SV = SQRT(32.2 * Y1(K) * S)	TRA 0270
	TAO = 1/8. * 1.9379 * FO * V1(K) * * 2.	TRA 0280
	DO 130 I = 1,NSIZ	TRA 0290
	QTV(K,I) = 0.	TRA 0300
	QBV(K,I) = 0.	TRA 0310
	QSV(K,I) = 0.	TRA 0320
C		TRA 0330
C	CALCULATE THE EXCESS SHEAR STRESS BY MEYER-PETER-MULLER	TRA 0340
C		TRA 0350
	TAC = 0.047 * (2.65 - 1) * 62.4 * DMB(I)	TRA 0360
	TTEM = TAO - TAC	TRA 0370
	IF (TTEM.LE.0.) GO TO 130	TRA 0380
C		TRA 0390
C	CALCULATE THE BED LOAD	TRA 0400
C		TRA 0410
	QBW = PN * TTEM * * 1.5	TRA 0420
	QBV(K,I) = PD(K,I) * XB(K) * QBW	TRA 0430
C		TRA 0440
C	CALCULATE THE SUSPENDED LOAD	TRA 0450
C		TRA 0460
	A = 2.0 * DMB(I)	TRA 0470
	IF (A.LT.0.05) A = 0.05	TRA 0480
	AR = A/Y1(K)	TRA 0490
	IF (AR.GT.0.5) GO TO 110	TRA 0500
	CA = QBW/(11.6 * A * SV)	TRA 0510
		TRA 0520

	C = - 6.0 * FVB(I)/(0.4 * SV * Y1(K))	TRA 0530
	EO = 1/C	TRA 0540
	EY = EXP(C * (Y1(K) - A))/C	TRA 0550
	QSW = CA * (EY - EO)	TRA 0560
	GO TO 120	TRA 0570
110	QSW = 0.	TRA 0580
120	QSV(K,I) = PD(K,I) * XB(K) * QSW	TRA 0590
C		TRA 0600
C	CALCULATE TOTAL BED MATERIAL LOAD	TRA 0610
C		TRA 0620
	QTV(K,I) = QBV(K,I) + QSV(K,I)	TRA 0630
	QSD(I) = QSD(I) + QTV(K,I)	TRA 0640
	QSK(K) = QSK(K) + QTV(K,I)	TRA 0650
130	CONTINUE	TRA 0660
	QSJ(J) = QSJ(J) + QSK(K)	TRA 0670
140	CONTINUE	TRA 0680
C		TRA 0690
C	CALCULATE THE AVERAGE CONCENTRATION FOR EACH SIZE FRACTION	TRA 0700
C		TRA 0710
C		TRA 0720
	RETURN	TRA 0730
	END	TRA 0740

```

SUBROUTINE PROPAR
C
C THIS SUBROUTINE DETERMINES THE PARTICLE SIZE DISTRIBUTION
C GIVEN THE PARTICLE SIZES OF INTEREST. IT ASSUMES A LOG
C NORMAL DISTRIBUTION.
C
COMMON FO , S , ND , NSIZ ,
1 Q(10) , FREQ(10) , QSJ(10) , X(30) ,
2 Z(30) , F(30) , Q1(30) , Y1(30) ,
3 XB(30) , V1(30) , XN(30) , DMB(10) ,
4 DBD(11) , FVB(10) , QSD(10) , QSDSUM(10) ,
5 RD(10) , RDSUM(30,10) , PD(30,10) , QSV(30,10) ,
6 QBV(30,10) , QTV(30,10) , EFS(30,10) , RATIO(30,10) ,
7 RATIOK(30) , QSK(30) , RKSUM(30) , RK(30) ,
8 D(30)
DIMENSION DLOG(15) , CDF(30,15)
C
NSP1 = NSIZ + 1
NDM1 = ND - 1
DO 100 I = 1,NSIZ
    DMB(I) = SQRT(DBD(I) * DBD(I + 1))
    DLOG(I) = ALOG(DBD(I))
100 CONTINUE
    DLOG(NSP1) = ALOG(DBD(NSP1))
    DO 130 K = 1,NDM1
        D50 = SQRT(D(K) * D(K + 1))
        SD = ALOG(2.0)
        DO 110 I = 1,NSP1
            Y = (DLOG(I) - ALOG(D50))/SD
            CALL MDNOR (Y,ERF)
            CDF(K,I) = ERF
110 CONTINUE
        DO 120 I = 1,NSIZ
            PD(K,I) = CDF(K,I + 1) - CDF(K,I)
120 CONTINUE
130 CONTINUE
C
RETURN
END

```

```

PRO 0010
PRO 0020
PRO 0030
PRO 0040
PRO 0050
PRO 0060
PRO 0070
PRO 0080
PRO 0090
PRO 0100
PRO 0110
PRO 0120
PRO 0130
PRO 0140
PRO 0150
PRO 0160
PRO 0170
PRO 0180
PRO 0190
PRO 0200
PRO 0210
PRO 0220
PRO 0230
PRO 0240
PRO 0250
PRO 0260
PRO 0270
PRO 0280
PRO 0290
PRO 0300
PRO 0310
PRO 0320
PRO 0330
PRO 0340
PRO 0350
PRO 0360
PRO 0370
PRO 0380
PRO 0390

```

SUBROUTINE BOX (J)

THIS SUBROUTINE CALCULATES THE AMOUNT OF SEDIMENT A SAMPLER
CATCHES AND THE SAMPLING EFFICIENTCY. THIS VERSION IS FOR A
VERTICLE BOX SAMPLER.

```
COMMON  FO      , S      , ND      , NSIZ      ,
1      Q(10)    , FREQ(10) , QSJ(10) , X(30)    ,
2      Z(30)    , F(30)   , Q1(30)  , Y1(30)   ,
3      XB(30)   , V1(30)  , XN(30)  , DMB(10)  ,
4      DBD(11)  , FVB(10) , QSD(10) , QSDSUM(10),
5      RD(10)   , RDSUM(30,10), PD(30,10) , QSV(30,10),
6      QBV(30,10), QTV(30,10), EFS(30,10) , RATIO(30,10),
7      RATIOK(30), QSK(30) , RKSUM(30) , RK(30)   ,
8      D(30)
```

DEFINE SAMPLER SIZE

WSP = SAMPLER WIDTH

HS = SAMPLR HEIGHT

DATA WSP/0.382/

DATA HS/1.5/

HS = 1.5

DSIN = WSP/2.

NDM1 = ND - 1

DO 200 K = 1,NDM1

SV = SQRT(32.2 * Y1(K) * S)

QSINK = 0.

QQSUMK = 0.

CALCULATE THE PERCENT OF SUSPENDED LOAD CAUGHT IN
THE SAMPLER.

DO 180 I = 1,NSIZ

IF (Q1(K).EQ.0.0) GO TO 150

IF (QTV(K,I).EQ.0.0) GO TO 160

IF (Y1(K).GT.HS) GO TO 100

RATIOH = 1.0

QQ = V1(K) * WSP * Y1(K)

GO TO 110

100 A = 2.0 * DMB(I)

IF (A.LT.0.05) A = 0.05

C = - 6.0 * FVB(I)/(0.4 * SV * Y1(K))

EO = 1.0/C

EHS = EXP(C * (HS - A))/C

EY = EXP(C * (Y1(K) - A))/C

RATIOH = (EHS - EO)/(EY - EO)

CALCULATE THE VOLUME OF WATER IN THE SAMPLE

QQ = V1(K) * HS * WSP

CALCULATE THE ENTRANCE EFFECTS

110	IF (DMB(I).GE.WSP) GO TO 120	BOX 0560
	IF (DMB(I).GT.DSIN) GO TO 130	BOX 0570
	RATIOE = 1.0	BOX 0580
	GO TO 140	BOX 0590
120	RATIOE = 0.	BOX 0600
	GO TO 140	BOX 0610
130	RATIOE = 1 - (DMB(I) - DSIN)/DSIN	BOX 0620
C		BOX 0630
C	CALCULATE THE SAMPLED SEDIMENT	BOX 0640
C		BOX 0650
140	Q SIN = RATIOE * (QBV(K,I)/XB(K) + RATIOH * QSV(K,I)/XB(K)) * WSP	BOX 0660
1		BOX 0670
	EFS(K,I) = (Q SIN/WSP)/(Q TV(K,I)/XB(K))	BOX 0680
	RATIO(K,I) = (Q SIN/QQ)/(QSD(I)/Q(J))	BOX 0690
	Q SINK = Q SINK + Q SIN	BOX 0700
	GO TO 180	BOX 0710
150	EFS(K,I) = - 1.0	BOX 0720
	QQ = 0.	BOX 0730
	IF (QSD(I).EQ.0.0) GO TO 170	BOX 0740
	RATIO(K,I) = 0.	BOX 0750
	GO TO 180	BOX 0760
160	EFS(K,I) = 1.0	BOX 0770
	IF (Y1(K).LT.HS) QQ = V1(K) * WSP * Y1(K)	BOX 0780
	IF (Y1(K).GE.HS) QQ = V1(K) * WSP * HS	BOX 0790
	IF (QSD(I).EQ.0.0) GO TO 170	BOX 0800
	RATIO(K,I) = 0.0	BOX 0810
	GO TO 180	BOX 0820
170	RATIO(K,I) = 1.0	BOX 0830
180	CONTINUE	BOX 0840
	QQSUMK = QQSUMK + QQ	BOX 0850
	IF (QQSUMK.EQ.0.0) GO TO 190	BOX 0860
	RATIOK(K) = (Q SINK/QQSUMK)/(Q SJ(J)/Q(J))	BOX 0870
	GO TO 200	BOX 0880
190	RATIOK(K) = 0.0	BOX 0890
200	CONTINUE	BOX 0900
	RETURN	BOX 0910
	END	BOX 0920

SUBROUTINE PUMP (J)

94

C					PUM 0010	
C					PUM 0020	
C	THIS SUBROUTINE CALCULATES THE AMOUNT OF SEDIMENT A SAMPLER				PUM 0030	
C	CATCHES AND THE SAMPLING EFFIENTCY. THIS VERSION IS FOR A				PUM 0040	
C	MULTIPLE PORT SAMPLER				PUM 0050	
C					PUM 0060	
	COMMON	FO	, S	, ND	, NSIZ	, PUM 0070
1		Q(10)	, FREQ(10)	, QSJ(10)	, X(30)	, PUM 0080
2		Z(30)	, F(30)	, Q1(30)	, Y1(30)	, PUM 0090
3		XB(30)	, V1(30)	, XN(30)	, DMB(10)	, PUM 0100
4		DBD(11)	, FVB(10)	, QSD(10)	, QSDSUM(10)	, PUM 0110
5		RD(10)	, RDSUM(30,10)	, PD(30,10)	, QSV(30,10)	, PUM 0120
6		QBV(30,10)	, QTV(30,10)	, EFS(30,10)	, RATIO(30,10)	, PUM 0130
7		RATIOK(30)	, QSK(30)	, RKSUM(30)	, RK(30)	, PUM 0140
8		D(30)				PUM 0150
C						PUM 0160
	DIMENSION		HPORT(10)			PUM 0170
C						PUM 0180
C	DEFINE SAMPLER					PUM 0190
C						PUM 0200
C	NPORT = NUMBER OF PORTS					PUM 0210
C	HPORT = HEIGHT OF PORT					PUM 0220
C	DIAPORT = DIAMETER OF PORT					PUM 0230
C						PUM 0240
	DATA NPORT/5/					PUM 0250
	DATA DIAPORT/0.02/					PUM 0260
	DATA HPORT/0.5,1.0,1.5,2.0,2.5/					PUM 0270
	DSIN = DIAPORT/2.					PUM 0280
	NDM1 = ND - 1					PUM 0290
C						PUM 0300
C	ITERATE OVER EACH SUBSECTION					PUM 0310
C						PUM 0320
	DO 230 K = 1,NDM1					PUM 0330
	SV = SQRT(32.2 * Y1(K) * S)					PUM 0340
	QSINK = 0.					PUM 0350
	QQSUMK = 0.					PUM 0360
C						PUM 0370
C	ITERATE OVER EACH SEDIMENT SIZE FRACTION					PUM 0380
C						PUM 0390
	DO 210 I = 1,NSIZ					PUM 0400
	QQSUM = 0.					PUM 0410
	QSIN = 0.					PUM 0420
	RATIOH = 1.0					PUM 0430
	IF (Q1(K).EQ.0.0) GO TO 180					PUM 0440
C						PUM 0450
C	ITERATE OVER EACH SAMPLING PORT					PUM 0460
C						PUM 0470
	DO 130 NP = 1,NPORT					PUM 0480
	IF (Y1(K).GE.HPORT(NP)) GO TO 100					PUM 0490
	RATIOH = FLOAT(NP - 1)/FLOAT(NPORT)					PUM 0500
	GO TO 140					PUM 0510
C						PUM 0520
C	CALCULATE THE VOLUME OF WATER IN THE SAMPLE					PUM 0530
C						PUM 0540
100	U = V1(K) + (SV/.4) * (ALOG(HPORT(NP)/Y1(K)) + 1.)					PUM 0550
	QQ = (DIAPORT * * 2/4.0) * U					PUM 0560
	QQSUM = QQSUM + QQ					PUM 0570

C		PUM 0580
C	CALCULATE THE SUSPENDED LOAD CAUGHT IN THE SAMPLER	PUM 0590
C		PUM 0600
	IF (QTV(K,I).EQ.0.0) GO TO 110	PUM 0610
	A = 2.0 * DMB(I)	PUM 0620
	IF (A.LT.0.05) A = 0.05	PUM 0630
	C = - 6.0 * FVB(I)/(0.4 * SV * Y1(K))	PUM 0640
	CA = QBV(K,I)/(11.6 * A * SV * XB(K))	PUM 0650
	CH = CA * EXP(C * (HPORT(NP) - A))	PUM 0660
	QSIN = QSIN + CH * QQ	PUM 0670
	GO TO 130	PUM 0680
110	EFS(K,I) = 1.0	PUM 0690
	IF (QSD(I).EQ.0.0) GO TO 120	PUM 0700
	RATIO(K,I) = 0.0	PUM 0710
	GO TO 130	PUM 0720
120	RATIO(K,I) = 1.0	PUM 0730
130	CONTINUE	PUM 0740
C		PUM 0750
C	CALCULATE THE ENTRANCE EFFECTS	PUM 0760
C		PUM 0770
140	IF (DMB(I).GE.DIAPORT) GO TO 150	PUM 0780
	IF (DMB(I).GT.DSIN) GO TO 160	PUM 0790
	RATIOE = 1.0	PUM 0800
	GO TO 170	PUM 0810
150	RATIOE = 0.	PUM 0820
	GO TO 170	PUM 0830
160	RATIOE = 1 - (DMB(I) - DSIN)/DSIN	PUM 0840
C		PUM 0850
C	CALCULATE THE SAMPLED SEDIMENT	PUM 0860
C		PUM 0870
170	QSIN = RATIOE * QSIN	PUM 0880
	EFS(K,I) = RATIOE * RATIOH	PUM 0890
	IF (QQSUM.EQ.0.0) GO TO 190	PUM 0900
	RATIO(K,I) = (QSIN/QQSUM)/(QSD(I)/Q(J))	PUM 0910
	QSINK = QSINK + QSIN	PUM 0920
	GO TO 210	PUM 0930
180	EFS(K,I) = - 1.0	PUM 0940
	QQSUM = 0.	PUM 0950
190	IF (QSD(I).EQ.0.0) GO TO 200	PUM 0960
	RATIO(K,I) = 0.	PUM 0970
	GO TO 210	PUM 0980
200	RATIO(K,I) = 1.0	PUM 0990
210	CONTINUE	PUM 1000
	QQSUMK = QQSUMK + QQSUM	PUM 1010
	IF (QQSUMK.EQ.0.0) GO TO 220	PUM 1020
	RATIOK(K) = (QSINK/QQSUMK)/(QSD(I)/Q(J))	PUM 1030
	GO TO 230	PUM 1040
220	RATIOK(K) = 0.0	PUM 1050
230	CONTINUE	PUM 1060
	RETURN	PUM 1070
	END	PUM 1080

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